

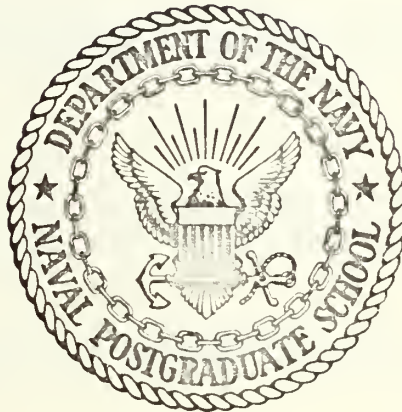
A GENERAL PURPOSE THREE DIMENSIONAL  
STRESS ANALYSIS PROGRAM

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# NAVAL POSTGRADUATE SCHOOL

Monterey, California



## THESIS

A GENERAL PURPOSE THREE DIMENSIONAL  
STRESS ANALYSIS PROGRAM

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December 1971

*Approved for public release; distribution unlimited.*





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Stress Analysis Program

by

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Submitted in partial fulfillment of the  
requirements for the degree of

MASTER OF SCIENCE IN MECHANICAL ENGINEERING

from the  
NAVAL POSTGRADUATE SCHOOL  
December 1971



## ABSTRACT

A computerized solution for three dimensional stress analysis was developed. The solution was based on the finite element technique employing twenty and thirty-two nodal point three dimensional isoparametric elements. The solution is applicable to linear elastic structures composed of homogeneous and isotropic materials subjected to static loading under constant temperature. Deformed states are restricted to small displacements and displacement gradients. The program is coded in standard FORTRAN IV and is machine independent except for one subroutine which may be adjusted to specific installations. Independence of core space requirements is achieved by use of direct access storage facilities.



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## I. INTRODUCTION

The finite element technique is a relatively new method for stress analysis of structural continua.

Due to its almost limitless abilities this technique has recently been extensively used in structural analysis and considerable research has been conducted toward the development of elements and techniques satisfying the requirements of advanced structural technology.

The present work concerns the development of a general purpose digital computer program for three dimensional stress analysis, named TRISOP, and employing the finite element method.

The program developed, in its present form, is applicable to the solution of problems concerning linearly elastic structures, composed of homogeneous and isotropic materials, and subjected to static loading at constant temperature. The deformed states are restricted to small displacements and displacement gradients. The use of an equation solver of, in principle, infinite capacity devised by Cantin [Ref. 2] in conjunction with direct access storage devices enabled the memory space requirements to be independent of the size of the problem, the latter being limited only by the execution time and availability of storage devices.

At the time of initiation of the development of TRISOP the most promising finite elements, in the three dimensional



analysis field, were the family of isoparametric elements. Two such elements, namely the 20 (Quadratic) and 32 (Cubic) nodal points elements, are used in the program.

The present form of TRISOP is an improved version of a program coded by Professor Gilles Cantin of the Naval Post-graduate School and employing quadratic isoparametric finite elements. The major improvements brought to this initial form of TRISOP by the present version are, the extension to cubic elements, inclusion of predefined displacements in the boundary conditions, improved execution time and independence from the size of the problem.

The structure of the program follows a modular pattern in order to provide for further improvements and, possibly, adjustment to new developments in the field.



## II. NATURE AND FORMULATION OF THE PROBLEM

The problem considered is a three dimensional stress analysis of a statically loaded structure under constant temperature. The analysis is restricted to deformed states within the elastic region and involving small displacement gradients. Consequently the linear strain tensor only need be considered and the strain-displacement relations can be expressed as:

$$\begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{xz} \\ \epsilon_{yz} \end{Bmatrix} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 & 0 \\ 0 & \frac{\partial}{\partial y} & 0 \\ 0 & 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial y} & \frac{\partial}{\partial x} & 0 \\ \frac{\partial}{\partial z} & 0 & \frac{\partial}{\partial x} \\ 0 & \frac{\partial}{\partial z} & \frac{\partial}{\partial y} \end{bmatrix} \begin{Bmatrix} u \\ v \\ w \end{Bmatrix} \quad (2.1)$$

or in more compact form:

$$\{\epsilon\} = [D]\{u\} \quad (2.2)$$

Where both  $\{\epsilon\}$  and  $\{u\}$  are point functions.

As a consequence of the above the principle of superposition applies to strains and displacements. The stress-strain law is assumed to be linear and thus the principle of superposition applies for stresses as well. Therefore,





displacements, strains and stresses of two distinct deformed states are algebraically additive resulting into a new compatible deformed state. Consequently initial stresses and strains need not be considered in the formulation of the problem. If such strains and stresses exist they will have to be superposed to the results of the solution which will be relative to a reference state, namely the state of the structure before the application of the load considered in the problem.

In addition to small displacement gradients, the analysis is restricted to displacements small enough that equilibrium and compatibility relations can be referred to the undeformed geometry of the structure (here and in what follows by undeformed state is meant the reference state).

Due to the restriction of small displacements the question of stability does not enter the analysis [Ref. 7]. The structure may be composed of more than one materials which are assumed to be homogeneous and isotropic. The constitutive laws for this problem can be expressed in matrix form as:

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{zz} \\ \sigma_{xy} \\ \sigma_{xz} \\ \sigma_{yz} \end{Bmatrix} = \frac{E^*}{(1+\mu)(1-2\mu)} \begin{bmatrix} 1-\mu & \mu & & & & \\ \mu & 1-\mu & & & & \\ \mu & \mu & & & & \\ & & \frac{1-2\mu}{2} & & & \\ & & & \frac{1-2\mu}{2} & & \\ & & & & \frac{1-2\mu}{2} & \end{bmatrix} \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \epsilon_{zz} \\ \epsilon_{xy} \\ \epsilon_{xz} \\ \epsilon_{yz} \end{Bmatrix}$$



or in more compact form:

$$\{\sigma\} = [E]\{\epsilon\} \quad (2.3)$$

Where  $\{\sigma\}$  is a point function,  $E^*$  is Young's modulus and  $\mu$  is Poisson's ratio.

#### A. FINITE ELEMENTS

The main feature of the finite element technique is the approximation of the elastic continuum possessing an infinite number of degrees of freedom by an assemblage of substructures, the finite elements, each possessing a finite number of degrees of freedom.

Among the various types, two elements of the isoparametric family were chosen for this development; namely, the 20 and 32 nodal point three dimensional elements. The concepts underlying the construction and use of these elements as well as the resulting properties will not be presented here except for those necessary for the presentation of the structure of the computer program. An extensive treatment of this family of finite elements as well as the isoparametric concept may be found in Refs. 4 and 8.

Two systems of reference are used throughout. First, a "local" non dimensional and, in general, non cartesian system  $(\xi, \eta, \zeta)$  is used for convenience of computation of the element stiffness matrices. Then everything is transferred in a "global" cartesian system of reference  $(x, y, z)$  for the final computations of the assembled system.



A numbering convention for the element nodes is employed and is shown in Figures 1 and 2. A different convention is used for numbering the nodes of the assembled structure and is described in Section IV.A.

By use of shape functions  $N_i(\xi, \eta, \zeta)$ , describing the variation of element properties in the local system, every element property  $\phi(\xi, \eta, \zeta)$  can be expressed in terms of its nodal values as:

$$\phi(\xi, \eta, \zeta) = \langle N_i \rangle \{ \phi_i \}$$

Thus the displacements of points within an element will be given by:

$$\begin{Bmatrix} u(x,y,z) \\ v(x,y,z) \\ w(x,y,z) \end{Bmatrix} = \begin{bmatrix} N_1 & 0 & 0 & N_2 & 0 & 0 & \dots \\ 0 & N_1 & 0 & 0 & N_2 & 0 & \dots \\ 0 & 0 & N_1 & 0 & 0 & N_2 & \dots \end{bmatrix} \begin{Bmatrix} u_1 \\ v_1 \\ w_1 \\ u_2 \\ v_2 \\ w_2 \\ \vdots \end{Bmatrix}$$

or in more compact form:

$$\{u\} = [N]\{u_i\}$$

An extensive treatment of the shape function concept may be found in Ref. 8.



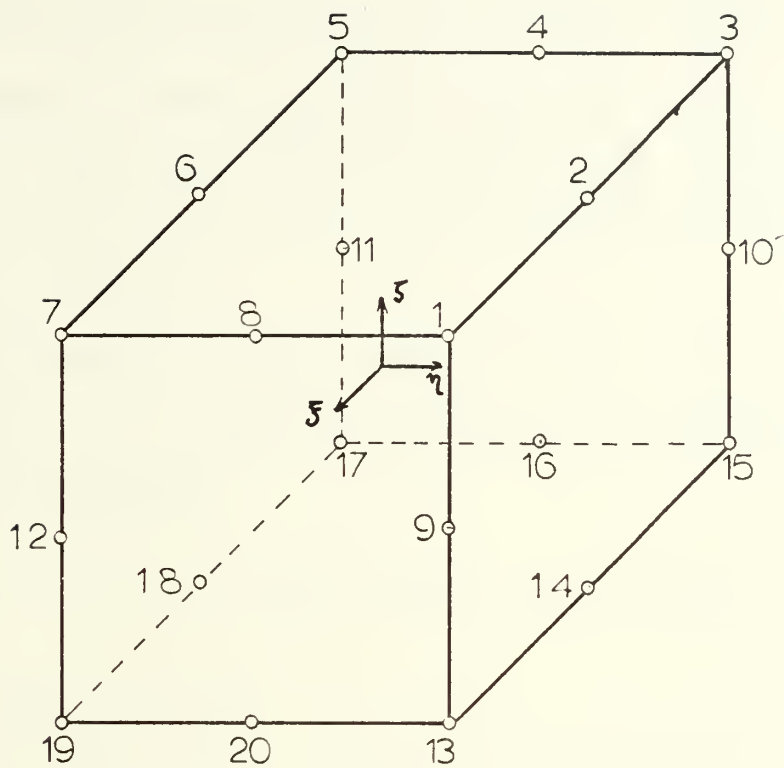


Figure 1.

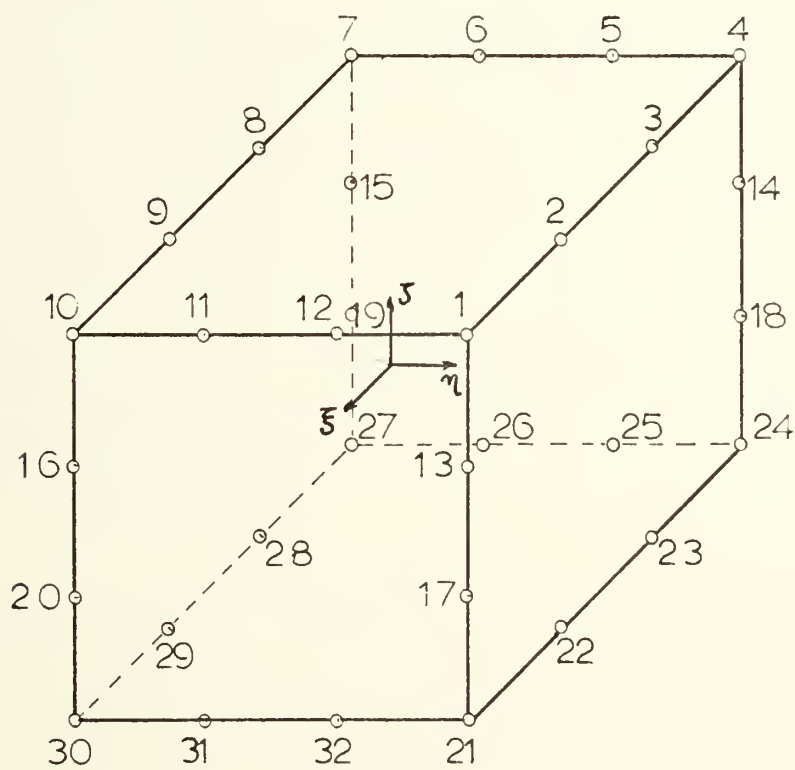


Figure 2.





The shape functions used in this development belong to the Serendipity family and are shown in Refs. 4 and 8.

#### B. ELEMENT AND TOTAL STIFFNESS MATRICES - CONSISTENT LOADS

Every individual finite element can be visualized as an independent structure and thus the derivation of a stiffness matrix is possible.

The derivation of the element stiffness matrix  $[K_e]$  is shown in Ref. 1 where the following integral expression is finally obtained:

$$[K_e] = \int_{-1}^1 \int_{-1}^1 \int_{-1}^1 [N^*]^T [E] [N^*] \det[J] d\xi d\eta d\zeta \quad (2.4)$$

where:  $[N^*] = [D][N]$

and  $[J]$  is the Jacobian matrix of the transformation.

This expression for the element stiffness matrix is ideally suited for Gaussian numerical integration. Computational considerations on the performance of this integration as well as the choice of integration points are presented in Ref. 1.

After formation of the stiffness matrix for each element, the total stiffness matrix  $[K]$  for the structure can be formed by superposition of corresponding stiffness coefficients.

The force-displacement relations for the whole structure can then be expressed as:

$$\{F\} = [K]\{u\} \quad (2.5)$$

where  $\{u\}$  is a vector containing all nodal displacements of the structure and  $\{F\}$  a vector of consistent concentrated loads applied at the nodes and equivalent to the loading applied to



the structure. The determination of the consistent loads is not incorporated in the present solution and it is assumed that it is known.

Relations (2.5) represent a system of linear equations. When this system is initially formed, both  $\{F\}$  and  $\{u\}$  will in general contain known and unknown elements. By further manipulation the system can be brought to the standard form with all unknowns on one side and its solution will result into the determination of all unknown nodal displacements and forces (see Section III.B.5).

### C. STRESSES AND STRAINS

By use of the strain-displacement relations (2.2) and the displacements obtained from the solution of system (2.5) the vectors of nodal strains can be determined.

However the application of relations (2.2) is only possible at the element level in the finite element theory. Thus the nodal strains for each element separately are first evaluated.

Further by applying the stress-strain relations (2.3) the element nodal strains are evaluated.

Except for nodes lying on the interfaces between elements of different materials, a stress analysis requires the determination of average nodal strains and stresses of all elements sharing common nodes. This is accomplished by simply averaging the algebraic values of the stress and strain components at each node.



### III. DESCRIPTION OF THE COMPUTER PROGRAM

Computerized solutions to structural problems based on the finite element technique have, in the recent years, led to a considerable amount of research in three disciplines: structural analysis, numerical analysis and computer programming. Thus new elements and computational techniques are continuously being developed paralleled by the development of computing equipment. For this reason it is not unusual for such computer programs to become obsolete within a few years of their completion.

Under these circumstances, the development of a successful program is required to follow rules which will preserve its effectiveness as long as possible by an adjustment process. These rules are summarized to:

- a. Standard coding and machine independence enabling the program to be used by various installations regardless of the computing equipment available, and
- b. Modular construction allowing adjustment to new developments without requiring the construction of the entire program.

These principles have governed the development of TRISOP at all stages that brought it to its present form.

The structure of TRISOP follows closely the analytic development of Section II. (Also see Flow Diagram.)



The program is coded in standard FORTRAN and is machine independent except for the subroutine DISK via which transfer of data to and from direct access storage devices is accomplished. All calculations are performed in double precision in order to provide adequate accuracy in spite of the round off errors.

The memory space requirements are independent of the size of the problem, the latter being limited only by the size of direct access storage space available and execution time.

The extension of the capabilities of the program, as for example inclusion of thermal stress analysis and nonhomogeneous materials will easily be accomplished without interfering with parts of the program that are not altered by these changes.

A number of checking points was incorporated in order to assure the proper development of the execution. Whenever required, error and/or warning messages are given in the output. If the continuation of the solution is useless due to the error detected the execution for this problem is stopped and the solution of the next problem (if such exists) begins.

The most important variables used throughout the program are shown in Appendix A and a brief description of their function is given.

The basic characteristics of the two programs, for 20 and 32 nodal point elements, are the same. The minor existing differences are discussed in Appendix G where listings of both programs are given.





In what follows the function of the different modules of the program is explained with reference to the analytic development of the solution as it is presented in Section II.

#### A. CONTROL PROGRAM

The control program serves no other purpose but to initiate the calling sequence of the different modules of the program and read, from input data cards, the characteristics (number and length of records) of the data files as these are requested by the JCL<sup>1</sup> cards. These characteristics are later on (in subroutine INPUT) checked for correctness and adequacy.

#### B. SUBROUTINE MAINE

Subroutine MAINE is the actual main program. It is coded in subroutine form in order to provide for the solution of more than one problem in one run by simply restarting the calling sequence of the different modules. Its function consists of a step by step calling procedure. The only numerical operation executed by subroutine MAINE itself is the computation of the average nodal stresses and strains after the element nodal stresses and strains have been computed by subroutine STRESS. In the coding of subroutine MAINE a timing procedure has been incorporated giving at the output the execution time for each step. This is an optional feature not connected with the execution of the problem itself and can be removed if required. However, it provides useful data for studying the factors affecting the execution time of a problem.

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<sup>1</sup> JCL = Job Control Language



The calling procedure and the function of the subroutines called are as follows:

1. Reading of Input Data

By calling subroutine INPUT all input data cards are read. First, the general characteristics of the problem (as number of elements, number of nodal points, etc.) are read and used for the computation of other characteristics (as total number of equations, number of blocks per column). All general characteristics of the problem are stored in a common storage area and used throughout the program by most of the subroutines. Subsequently the arrays of connectivity, nodal point coordinates, material properties, concentrated load data, and boundary conditions are read. All arrays except for the material properties (which is of pre-specified size) are stored in data files on direct access storage devices. This feature enables the program to be independent of the size of the problem. The bandwidth and the number of blocks per row of the total stiffness matrix are computed from connectivity data. All input data as well as those computed are printed out for checking purposes.

The nature of the program allows the solution of a great variety of problems. For this reason and in order to avoid limitations on the form of the input data most reading formats are not specified but left to the choice of the user by use of object time formats.

Within subroutine INPUT several checks are made in order to insure the consistency of some data. All such checks



throughout the program are accomplished by calling the proper entry of subroutine ERROR. The checks made by subroutining INPUT are the following.

a. Stop Trap for the Last Problem

The last problem of each run is followed by an indicator card, as explained in Section IV. Once this indicator card is read, the execution is stopped.

b. Number of Different Materials

The maximum permissible number of different materials present in the structure to be analyzed is ten. If a problem involves a number of materials greater than ten, the execution is stopped and an error message is given in the output.

c. Characteristics of Data Files

The characteristics of data files, namely the number and size of records for each file, are given to the program once for every run in the main program but their consistency is checked for every problem of the run. If the size of one or more of these characteristics is smaller than that required for the problem the execution is stopped for this problem and execution of the next problem (if such exists) begins. In case the size of one of the data files characteristics exceeds that required for the problem, a warning message is given at the output and execution continues.

2. Formation of Element Nodal Coordinate Arrays

The coordinates of nodal points are provided to the program via subroutine INPUT in a sequence following the node



numbering convention for the whole structure. However, at various points of the program the use of arrays containing the nodal coordinates of each individual element is required. The formation of such arrays is accomplished by subroutine SORT. The sequence of elements of these arrays follows the numbering convention of the element nodes. Each array formed is stored in a direct access data file.

### 3. Formation of Element Stiffness Matrices

The individual element stiffness matrices  $[K_e]$  are formed by subroutine FSTF in the following way.

The element stiffness matrix is given by the expression

$$[K_e] = \int_{-1}^1 \int_{-1}^1 \int_{-1}^1 [N^*]^T [E] [N^*] \det[J] d\xi d\eta d\zeta \quad (3.1)$$

First the matrix of elastic constants  $[E]$  is computed by calling subroutine ELPROP. In case the whole structure is made of one material this operation is executed only once since all elements have a common elastic matrix. In the case of more than one material matrix  $[E]$  is computed every time the element considered is of different material than the previous one. Subsequently subroutine CUB is called to perform Gaussian numerical integration. For each Gauss point subroutine FORMK is called by CUB and the integrant of expression (3.1) is formed in a step by step procedure and evaluated at the integration point. Once an element stiffness matrix is formed it is stored on disk.





#### 4. Formation of Total Stiffness Matrix

The formation of the total stiffness matrix is performed in subroutine MERGE by superposition of the corresponding stiffness coefficients. In order to avoid the presence of the whole stiffness matrix in core, it is partitioned in blocks of pre-specified size. These blocks are formed one at a time by selecting the proper elements from the element stiffness matrices. Each element stiffness matrix is scanned in order to detect elements belonging to the block under formation. Every completed block of the total stiffness matrix is in turn stored on disk. The method of partitioning and storing of the total stiffness matrix is established in Ref. 2. A thorough study of this paper is essential in order to comprehend the procedure outlined here as well as to become familiar with the program itself.

#### 5. Formation of Load Vector and Application of Boundary Conditions

After the formation of the total stiffness matrix the completion of system 2.5 requires the formation of vectors  $\{F\}$  and  $\{u\}$  and some further manipulation of the system in order to bring it in the standard form.

The load vector  $\{F\}$  is partitioned into a number of smaller vectors the size of which corresponds to the size of total stiffness matrix blocks. Each such vector is formed in sequence by use of the consistent load data, stored on disk from the first step of the execution (subroutining INPUT), and subsequently is stored on disk. Load vector elements



corresponding to unknown forces, namely reactions at the constraints, are replaced at this stage by zeros.

The boundary conditions imposed to the problem result in the presence of a number of unknown reactions, designated hereby  $F_i^*$ , and predefined displacements, designated by  $u_i^*$ . If such a boundary condition is applied to the  $i$ th element of the force and displacement vectors the system will have the form:

$$\begin{Bmatrix} F_1 \\ \vdots \\ F_i^* \\ \vdots \\ F_n \end{Bmatrix} = \begin{bmatrix} K_{11} & \dots & K_{1i} & \dots & K_{1n} \\ \vdots & & \vdots & & \vdots \\ \vdots & & \vdots & & \vdots \\ K_{i1} & \dots & K_{ii} & \dots & K_{in} \\ \vdots & & \vdots & & \vdots \\ \vdots & & \vdots & & \vdots \\ K_{n1} & \dots & K_{ni} & \dots & K_{nn} \end{bmatrix} \begin{Bmatrix} u_1 \\ \vdots \\ u_i^* \\ \vdots \\ u_n \end{Bmatrix},$$

This system is equivalent to a good degree of approximation to the system

$$\begin{Bmatrix} F_1 - K_{1i}u_i^* \\ \vdots \\ 0 \quad K_{ii}u_i^* \\ \vdots \\ F_n - K_{ni}u_i^* \end{Bmatrix} = \begin{bmatrix} K_{11} & \dots & K_{1i} & \dots & K_{1n} \\ \vdots & & \vdots & & \vdots \\ \vdots & & \vdots & & \vdots \\ K_{i1} & \dots & B & \dots & K_{in} \\ \vdots & & \vdots & & \vdots \\ \vdots & & \vdots & & \vdots \\ K_{n1} & \dots & K_{ni} & \dots & K_{nn} \end{bmatrix} \begin{Bmatrix} u_1 \\ \vdots \\ F_i^{**} \\ \vdots \\ u_n \end{Bmatrix}$$

Where  $B$  is a number of several orders of magnitude greater than the greater stiffness coefficient and  $F_i^{**} = -\frac{F_i^*}{B}$

The products  $F_i^{**}K_{ii}, \dots, F_i^{**}K_{ni}$  are negligible in size compared to the other coefficients. After the system is solved the unknown reaction  $F_i^*$  is determined by:  $F_i^* = -BF_i^{**}$



This is a well known method of solution of mixed systems and it is felt that no further explanation is required [Ref. 5].

The process of application of the boundary conditions and transformation of the system to a form suitable for solution is performed by subroutine BCOND. In the case where all predefined displacements are equal to zero the process can be considerably simplified. In this case the final form of the system is:

$$\begin{Bmatrix} F_i \\ \vdots \\ 0 \\ \vdots \\ F_n \end{Bmatrix} = \begin{bmatrix} K_{11} & \dots & K_{1i} & \dots & K_{1n} \\ \vdots & & \vdots & & \vdots \\ \vdots & & \vdots & & \vdots \\ K_{i1} & \dots & B & \dots & K_{in} \\ \vdots & & \vdots & & \vdots \\ \vdots & & \vdots & & \vdots \\ K_{n1} & & K_{ni} & & K_{nn} \end{bmatrix} \begin{Bmatrix} u_1 \\ \vdots \\ F_i^{**} \\ \vdots \\ u_n \end{Bmatrix}$$

It is apparent that the only operation required is the substitution of the diagonal element  $K_{ii}$  by  $B$ .

For this reason subroutine BCOND was provided with two entries, one for the case of all zero predefined displacements and one (BCOND2) for the case of one or more nonzero predefined displacements. A number  $B=10^{20}$  is used and is considered sufficient for the purpose of the program. The operations are carried out in a step by step procedure in a sense that at maximum two stiffness matrix coefficient blocks and their corresponding load vectors are simultaneously present in core.



## 6. Solution of the System - Determination of Reactions

The solution of the system is performed by subroutine SOLVE. The technique employed was devised by Professor Gilles Cantin of the Naval Postgraduate School and presented in Ref. 4.

In the process of solving the system of equations the inversion of diagonal blocks of the stiffness matrix is required. This inversion is performed by subroutine SYMINV. Before inversion of the block is performed it is checked whether it is singular or nearly singular. The trace of the matrix is evaluated (excluding diagonal elements containing the number  $B=10^{20}$ ) and all diagonal elements are compared to the trace multiplied by a factor of  $10^{-12}$ . If a diagonal element is found smaller than this product the block is characterized as nearly singular. The execution continues but a proper indication is given by SYMINV to SOLVE which in turn gives a warning message at the output via subroutine ERROR. The choice of the factor  $10^{-12}$  was based on the number of significant digits provided by an IBM/360 machine on which the program was tested. Should the program be used on a different machine, alteration of this number should be considered in a sense that the difference between the exponent of the factor and the number of significant digits provided by the machine should be adequate to provide for loss of accuracy due to round off errors. An extensive presentation of the concepts underlying this idea is beyond the scope of this work.





If one diagonal element is found to be equal to zero the block is singular and further continuation of the execution is impossible. Again a proper indication is given by SYMINV to SOLVE which in turn gives an error message at the output via subroutine ERROR and the execution is stopped for this problem. Consequently the calling procedure is restarted from the beginning and the execution of the next problem begins (if such exists).

Upon inversion of each diagonal block  $[A]$  of the stiffness matrix a condition number  $C$  is evaluated for this block within SYMINV. This condition number is defined as  $C = \|A\| \cdot \|A^{-1}\|$  whereby  $\|A\|$  is meant a norm of  $[A]$ . This condition number is printed out, and provides an upper bound on the expected relative error of the results of the problem. The difference between the number of significant digits provided by the machine used and the maximum exponent of the condition number gives approximately the number of significant digits in the displacement results. Further information on the significance of the condition number can be found in Ref. 6.

Following the solution of the system of equations the reactions are evaluated by subroutine DISP as explained in Section B5 but with an opposite sign. Consequently the nodal displacements and reactions are printed out by DISP.

## 7. Evaluation of Nodal Strains and Stresses

The nodal displacements as found by the solution of the system 2.5 are stored in vector form and in a sequence following the numbering convention of the nodes for the whole structure (Section II.A).



The determination of element nodal displacements by use of relation 2.2 requires a displacement vector containing the displacement components of all nodes of the corresponding element in a sequence following the numbering convention for individual elements (Section II.A). The formation of such vectors is accomplished by properly sorting the displacement components via subroutine DISK (entry WDISKB) called by DISP.

Subroutine STRESS evaluates the element nodal strains and stresses by use of relations 2.2 and 2.3. The evaluation of the product  $[D]\{u\}$  requires the use of subroutine FORMK. The computational details of this operation are explained in Ref. 1.

Printing of element strains and stresses is optional since they provide useful information only for nodes on interfaces between elements of different materials.

Evaluation and printing of average nodal strains and stresses is performed by subroutine MAINE and is the concluding operation in the solution of each problem.

#### C. SUBROUTINE DISK

Transfer of data to and from the data files stored on direct access storage devices is accomplished by subroutine DISK. This subroutine uses multiple entries, each entry performing a different operation. Calling of DISK has been reduced to a minimum to avoid useless expense of execution time in data transfer. The most critical entries are those used for transferring element stiffness matrices and total stiffness matrix

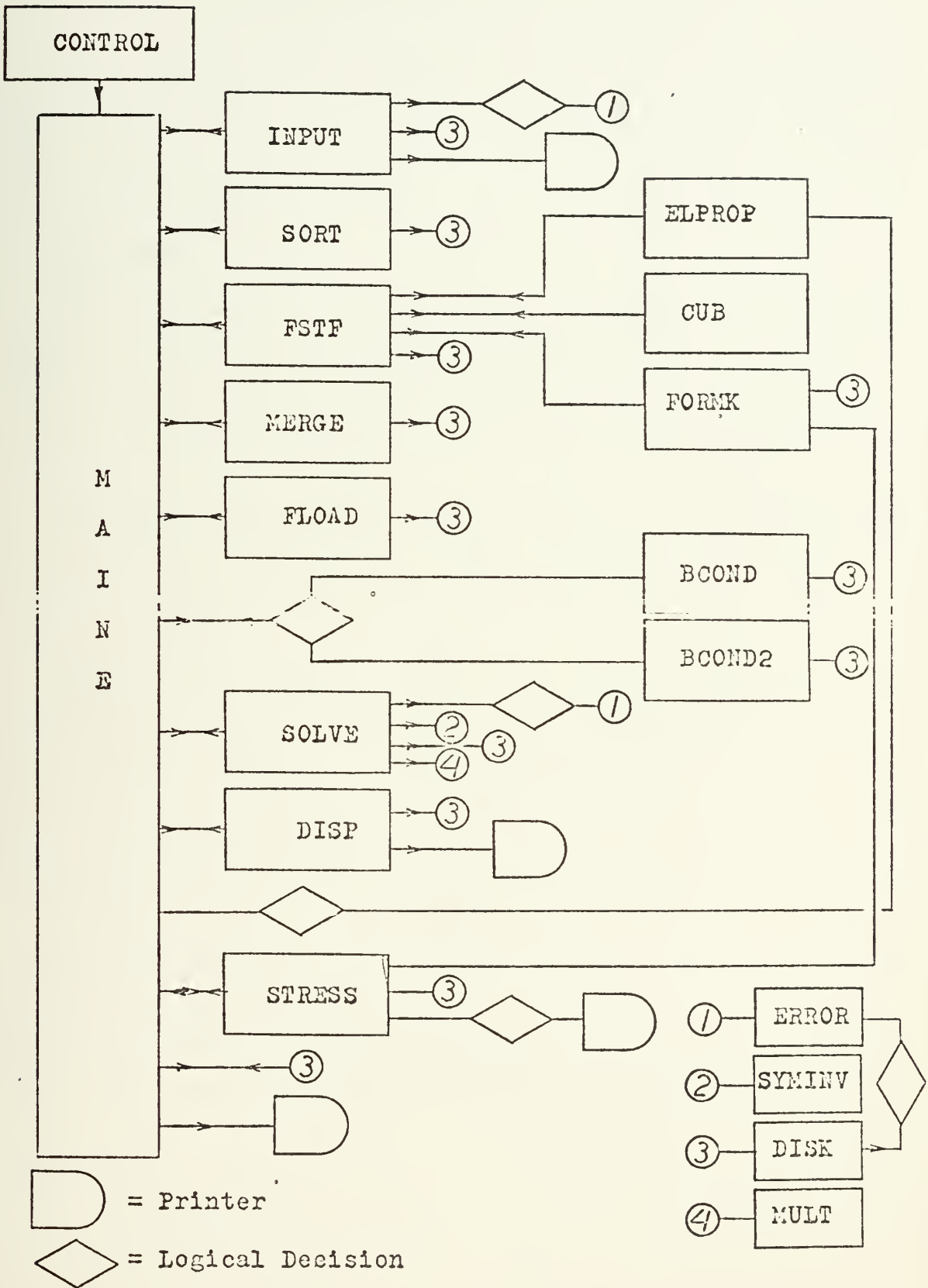


blocks. The reason being the size of the arrays transferred and the number of transfers required. Reduction of the transfer time requires the use of the maximum possible record length. The size of element stiffness matrices is known for each type of element, thus the optimum record length is determined and the corresponding entries are coded on this basis. However, the block size in the equation solver is determined by the user and thus the optimum record length must be determined by the user (see Section IV). The function of the various entries is explained below. Entry names starting with the letter W denote transfer of data to a data file and names starting with the letter R denote transfer of data from data files to core.

RDDISK, WRDISK:	File number 7. Transfer of total stiffness matrix blocks.
RDISK1, WDISK1:	File number 8. Transfer of element stiffness matrices.
RDISKC, WDISKC:	File number 10. Transfer of nodal coordinate component vectors COORD (3)
RDSKR1, WDSKR1:	File number 20. Transfer of load vector blocks when stored in work vector RB1. Similarly for RDISKR2, RDISKR3, WDISKR2, WDISKR3.
RDISKB, WDISKB:	File number 21. Transfer of individual node displacement components vectors. The random access data file thus formed is used for the formation of element nodal displacement vectors.
RDISKS, WDISKS:	File number 12. Transfer of vector SSSS1 (See Appendix A.)



## FUNCTIONAL FLOW DIAGRAM







#### IV. PROBLEM AND INPUT DATA PREPARATION

In this section the various steps in modeling a problem to be solved and the preparation of input data for the computer program are described.

##### A. STRUCTURE MODELING

The first step in applying a finite element solution to a problem is the selection of an appropriate mesh. In many cases an extrapolation of the results will be required and hence more than one meshes will have to be selected.

The mesh size selection does not follow any predetermined rules and will, in general, depend on the nature of the problem and the judgement of the analyst.

An arbitrary numbering convention must be adopted for the nodal points of the entire structure (in distinction with the numbering convention for the element nodes shown in Section II). The relation of node numbers in the two numbering systems establishes the connectivity matrix described in Section IV.B.1.

Though no restriction is imposed on the selection of the numbering convention for the entire structure, it has been shown in practice that an intelligent selection may considerably reduce the bandwidth of the total stiffness matrix.



## B. INPUT DATA PREPARATION

Another factor to be determined by the analyst is the size of the blocks of coefficients in the equation solver. The block size affects the execution time required for solution of the system of equations by subroutine SOLVE. Some experimentation has been conducted on this subject by Cantin and is presented in Ref. 2. On the basis of this experimentation it seems that the larger the block size the lesser the execution time is. However, the subject has not, as yet, been thoroughly investigated and no firm rules can be stated at present. TRISOP allows the use of any block size. However, if a block size smaller than the size of the element stiffness matrix is selected, the dimensions of the various arrays of common block B3 may have to be changed.

After selection of the block size the number of records per block NREC7 and record length of file number 7 can be determined. Maximum transfer efficiency requires that NREC7 is minimum and thus the record length the maximum possible for the machine used. Subsequently the remaining input data can be determined. In the following a description of the non self-explanatory data is given.

### 1. Connectivity Matrix

The size of this matrix is  $NEL \times NPEL$ . Each row corresponds to one element and represents the correspondence of node numbers in the two numbering systems, namely, the element node numbering system and the structure node numbering



system. The values of the elements of each row are the node numbers in the entire structure system and are sequenced according to the element node numbering system. Thus, the second element of the fifth row represents node number two of finite element number five and its value is the number of this node in the structure system.

## 2. Nodal Point Coordinates

The nodal point coordinates are defined in the global system of reference. Cartesian or cylindrical coordinates may be used. These coordinate systems are defined in Fig.

3. All three coordinates of each node must be defined in the

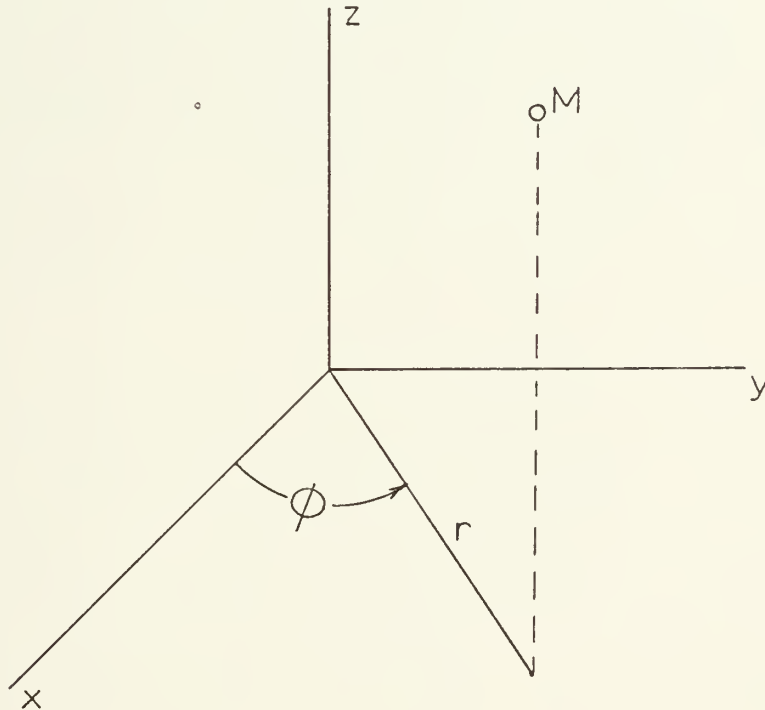


Figure 3.



same system. However, different nodes may be defined in different coordinate systems. Each set of nodal coordinates must be accompanied by a code number (KND) indicating whether cartesian or cylindrical coordinates are used.

### 3. Material Properties

The maximum permissible number of different materials is ten. In case the structure is composed of more than one materials a code number (NCON21) must be assigned to each material (NCON21=1,2,..., n, where n is the number of materials present in the structure). Material properties are given in the form of matrix ELDAT. Each row represents a different material and its elements are in sequence, Young's modulus, Poisson's ratio and coefficient of thermal expansion. The present form of TRISOP does not provide for thermal stress analysis and thus no value is required to be given to the coefficient of thermal expansion.

### C. DATA CARDS

In order to preserve the flexibility of the input no reading formats are specified in the program except for titles, object time format arrays and general characteristics of the problem. Object time formats, stored in the array FMT, are used for reading input data. The same formats are used for echo-printing of the input data. In the description of input data cards the variable FMT denotes the object time format under which the corresponding data will be read. One data card is required per format. Formats may be arbitrarily





specified by the user. However it is recommended that only one data card be used for the variables described in Sections IV.C.5.c; 6.c; 7.c; 8.c; 9.c and 10.c. The number of cards required per group of data is based on the above recommendation.

Every group of data cards is preceded by a card containing alphanumeric information. Exception is made only for the connectivity matrix of cubic elements where two such cards are required. The contents of these cards are stored in the array TITLE and appear as identification headings at the output.

Both TITLE and FMT are read under a (10A8) format.

#### 1. Characteristics of Data Files

These data are given only once for each run and are read under a (7I10) format. Six cards are required on which the values of record length and number of records for the various data files are given in the sequence: NR7, LRW7, NREC7, NR8, LRB8, LRW8, NR9, LRB9, NR10, LRB10, LRW10, NR12, LRB12, LRW12, NR13, LRB13, LRW13, NR14, LRB14, NR15, LRB15, NR16, LRB16, NR17, LRB17, NR19, LRB19, NR20, LRB20, LRW20, NR21, LRB21, LRW21, NR22, LRB22.

These cards can be produced by use of the auxiliary program JCL as described in Appendix B. The required values of the above variables are given in the same Appendix.

#### 2. Initiation Card

Format: 1A8

Number of cards per problem: one

The word START must be punched starting at column one.



### 3. Problem Identification

Format: (10A8)

Number of cards per problem: one

Any alphameric information identifying the problem and punched in columns 2-80.

### 4. General Characteristics of the Problem

Format: (10I5)

Number of cards per problem: one

<u>Columns</u>	<u>Variable</u>	<u>Meaning</u>
1-5	NEL	Total number of elements
6-10	NDPT	Total number of nodal points
11-15	NS	Stiffness matrix block size
16-20	NPEL	Number of nodes per element
21-25	NMAT	Number of different materials
26-30	NCLD	Total number of nodes with concentrated loads
31-35	NPBC	Total number of nodes with boundary conditions
36-40	NPBC2	Number of nodes with non zero predefined displacements
41-45	NLINE	Number of lines to be printed per page for strain and stress tables
46-50	KEL	KEL=0: No printing of element nodal strains and stresses is required. KEL≠0: Printing of above is is required.

### 5. Connectivity Matrix

a. TITLE

b. FMT

c. Connectivity data.



Number of cards: NEL

Variables: NCON= Array of connectivities for one  
element (one row of connectivity  
matrix)

NCON21= Material code number for the  
corresponding element (if required)

The connectivity matrix is read row-wise one row at a  
time. In the case of structures composed of more than one  
material, the last element of NCON must be followed by NCON21.

## 6. Nodal Coordinates

- a. TITLE
- b. FMT
- c. Coordinate data

Number of cards: NDPT

Variables: IEL= Node number in the entire struc-  
ture numbering system.

COORD= Array of coordinates of node IEL  
in the global system. Coordinates  
are given in the sequence x, y, z  
or r,  $\phi$ , z.

KND= Code number indicating cartesian  
or cylindrical coordinates.

KND=0: Cartesian coordinates

KND $\neq$ 0: Cylindrical coordinates.

## 7. Material Properties

- a. TITLE
- b. FMT



c. Material property data

Number of cards: NMAT (maximum ten)

Variables: N= Material code number

ELDAT= Array of material properties.

8. Nodal Consistent Load Data

a. TITLE

b. FMT

c. Load data

Number of cards: NCL= Node number in the numbering  
system of the entire struc-  
ture

CLOAD1= Array of concentrated load  
components in the global  
system. Load components  
are given in the sequence  
x,y,z.

Load data cards must be in ascending order of NCL  
values.

9. Boundary Condition Codes

a. TITLE

b. FMT

c. Codes

Number of cards: NPBC

Variable: NBC= Array of boundary condition codes  
for each node. The first element  
is the node number and the remain-  
ing three represent the boundary





conditions applied in the x,y,z  
direction respectively.

The codes employed are:

0: Free node.

1: Node constrained to zero  
displacement.

2: A predefined displacement  
exists.

The boundary condition code data cards must be  
sequenced in ascending order of node number.

10. Predefined Displacements (if required)

a. TITLE

b. FMT

c. Displacement data.

Number of cards: NPBC2

Variables: NDBC = Node number.

BCON = Array of predefined displacement  
components in the global system,  
given in the x,y,z sequence.

Predefined displacement data cards must be sequenced in  
ascending order of the values of NDBC.

In case no predefined displacements exist in a problem  
(NPBC2=0) all cards described in this paragraph must be omitted.

11. Subsequent Problems - Stop Trap for Last Problem

In case more than one problem is to be executed in  
one run, the data information described in paragraphs IV.C,2  
to IV.C.10 must be provided for each problem. The initiation



card (START) of each problem must immediately follow the last card of the previous problem.

The last problem must be followed by the following cards which serve as a stop trap.

- a. One card with the word "START" starting in column one.
- b. One card containing any alphanumeric information (or blank).
- c. One card with any negative integer punched in columns 1-5.

In order to provide for a better understanding of the above documentation sample inputs for two problems, one utilizing quadratic and one cubic elements are presented in Appendix C.

#### D. DIRECT ACCESS DATA FILES REQUIREMENTS

The method of specification for the direct access file requirements will vary from one installation to the other. The particular features of the different installations will affect the form of subroutine DISK and the JCL cards. In Appendix B these requirements are described for the IBM 360/67 operating system. The space requirements for each file as described in Appendix C are those to be used in establishing the form of subroutine DISK and JCL cards for any machine.



## V. SOLUTION OF A SIMPLE PROBLEM

When the initial form of TRISOP, for quadratic elements, was coded by Professor Cantin, it was tested on simple problems, and results were obtained and compared with classical solutions.

Testing of the present development of TRISOP was based on these problems and no further experimentation was conducted.

In order to show the function of the program, as well as some observations affecting the use of it, the solution of a simple problem is presented in this section.

The problem selected is that usually presented in classical texts of strength of materials as a simply supported beam of uniform cross section subjected to a concentrated load in the middle of the span.

The model used for this problem for the quadratic element formulation is shown in Fig. 4. The concentrated load is substituted by a line load. Nodes 1, 2 and 3 (here arbitrarily numbered) are constrained to zero displacement in all directions and nodes 4, 5 and 6 are allowed to move in the  $z$  direction only. The model for cubic elements is the same with the difference that the constrained nodes are four at each end instead of three.

The consistent loads corresponding to the line load applied were found as required by the program. Such loads appear only at the nodes of the midsection of the beam and are shown in Fig. 6.



Five different meshes were employed in order to show the variation of results with mesh size. The meshes chosen were 2, 4, 6, 8 and 10 elements in the z direction for both quadratic and cubic elements.

Numerical integration was carried by use of 2, 3 and 4 Gauss points (in different runs) for quadratic elements and 3, 4 and 5 points for cubic elements. Listings of the subroutines used are shown in Appendix I.

Integration with 3 and 4 Gauss points for quadratic elements and 4 and 5 points for cubic elements gave essentially the same results for this problem. For problems using elements with curved boundaries the results will vary with the number of Gauss points used. The solution with quadratic elements using 2 integration points was numerically unstable and no results were obtained. Numerical results for the maximum displacement at the midsection are given in Tables I and II. The same results are shown graphically in Fig. 7 for both quadratic and cubic elements. In the same figure, the execution time for subroutine FSTF and SOLVE, which consume most of the execution time for each problem, is shown. Figure 8 shows graphically the displacements and execution time obtained from the solution using cubic elements.

#### A. ALTERNATE FORMULATION

In order to observe some further features of the finite element solution, the same problem was solved formulated in a different way.





By use of the existing symmetry in the  $z$  direction, only half of the beam was considered as shown in Fig. 5. All nodes lying in the plane of symmetry were constrained to a zero displacement in the  $x$  direction. The presence of additional constraints enabled the use of two integration points. Numerical values for the maximum displacement in the midsection are shown in Table III. The same results as well as execution time are shown in graphical form in Fig. 9.

## B. DISCUSSION OF RESULTS

Observation of the results presented here reveals that mesh refinement leads to improved results which eventually will converge to a certain value. Use of less accurate numerical integration by reducing the number of integration points leads to improved results and considerable reduction of execution time. Also the use of existing symmetry and in general different formulations of the problem may prove to be beneficial.

Results obtained by use of cubic elements are considerably better than those of quadratic elements. However the execution time is appreciably higher for cubic elements and it is a matter of judgement whether cubic or quadratic elements should be used for each specific problem.

These are observations essentially made on the results of a single and basically simple problem. Thus no firm rules regarding the use of TRISOP can be established and further experimentation with different problems has to be conducted for this purpose.



TABLE I

Quadratic Elements - Maximum Displacements in  $\times 10^{-4}$

ELEM.	2	4	6	8	10
CUB 3 or CUB 4	5.761	6.847	6.992	7.026	7.038

TABLE II

Cubic Elements - Maximum Displacements in  $\times 10^{-4}$

ELEM.	2	4	6	8	10
CUB 5 or CUB 4	7.045	7.055	7.062	7.067	7.070
CUB 3	7.045	7.067	7.068	7.069	7.072

TABLE III

Quadratic Elements - Alternate Formulation

Max. Disp. in  $\times 10^{-4}$

ELEM.	2	4	6
CUB 4 or CUB 3	5.761	6.847	6.992
CUB 2	7.191	7.066	7.053



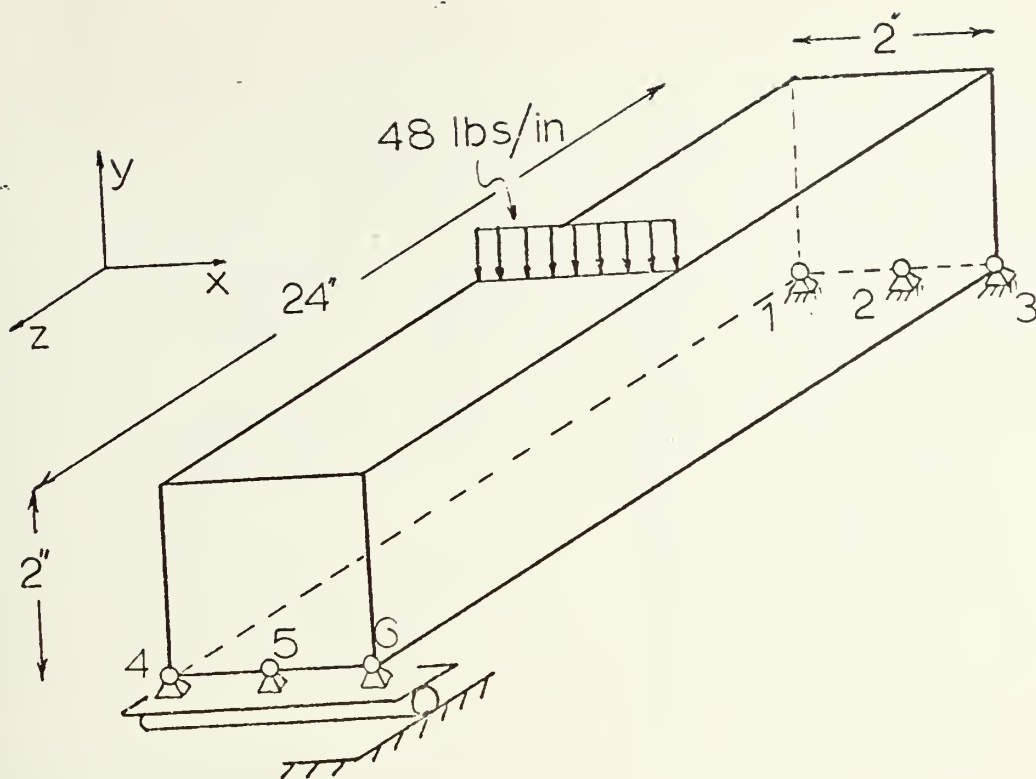


Figure 4

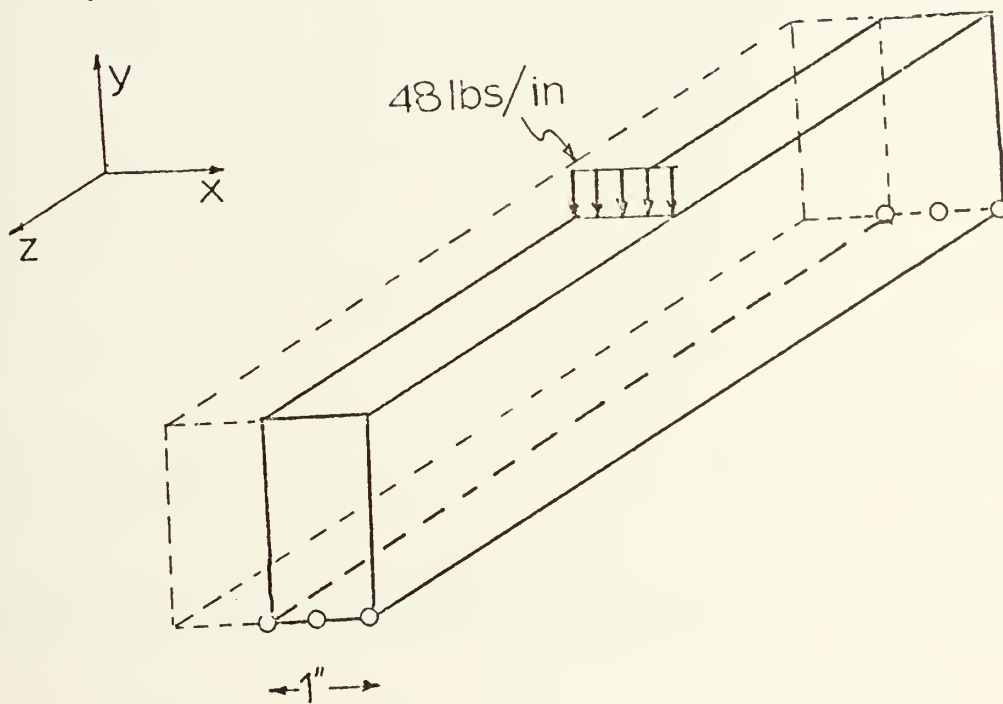
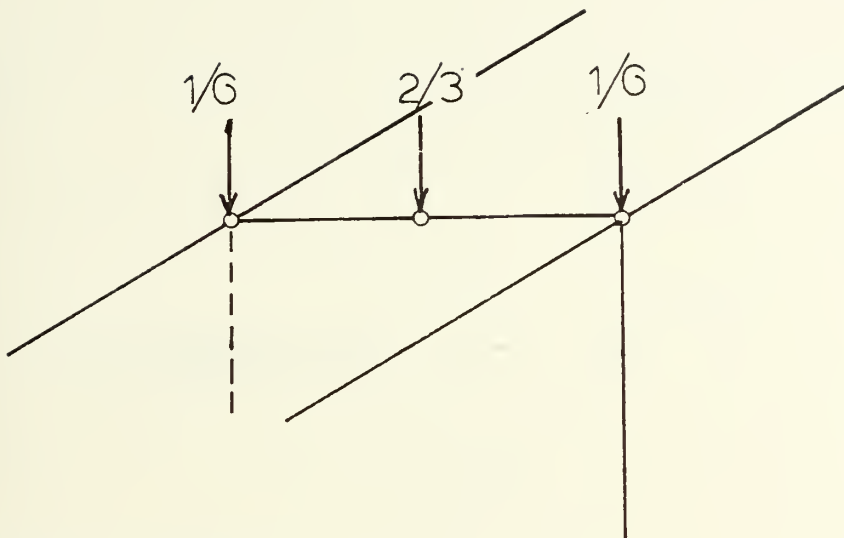
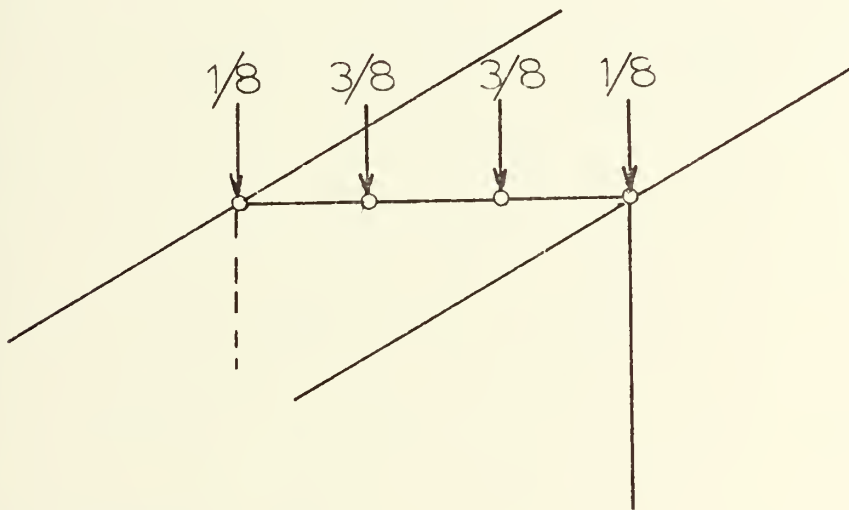


Figure 5





Quadratic elements



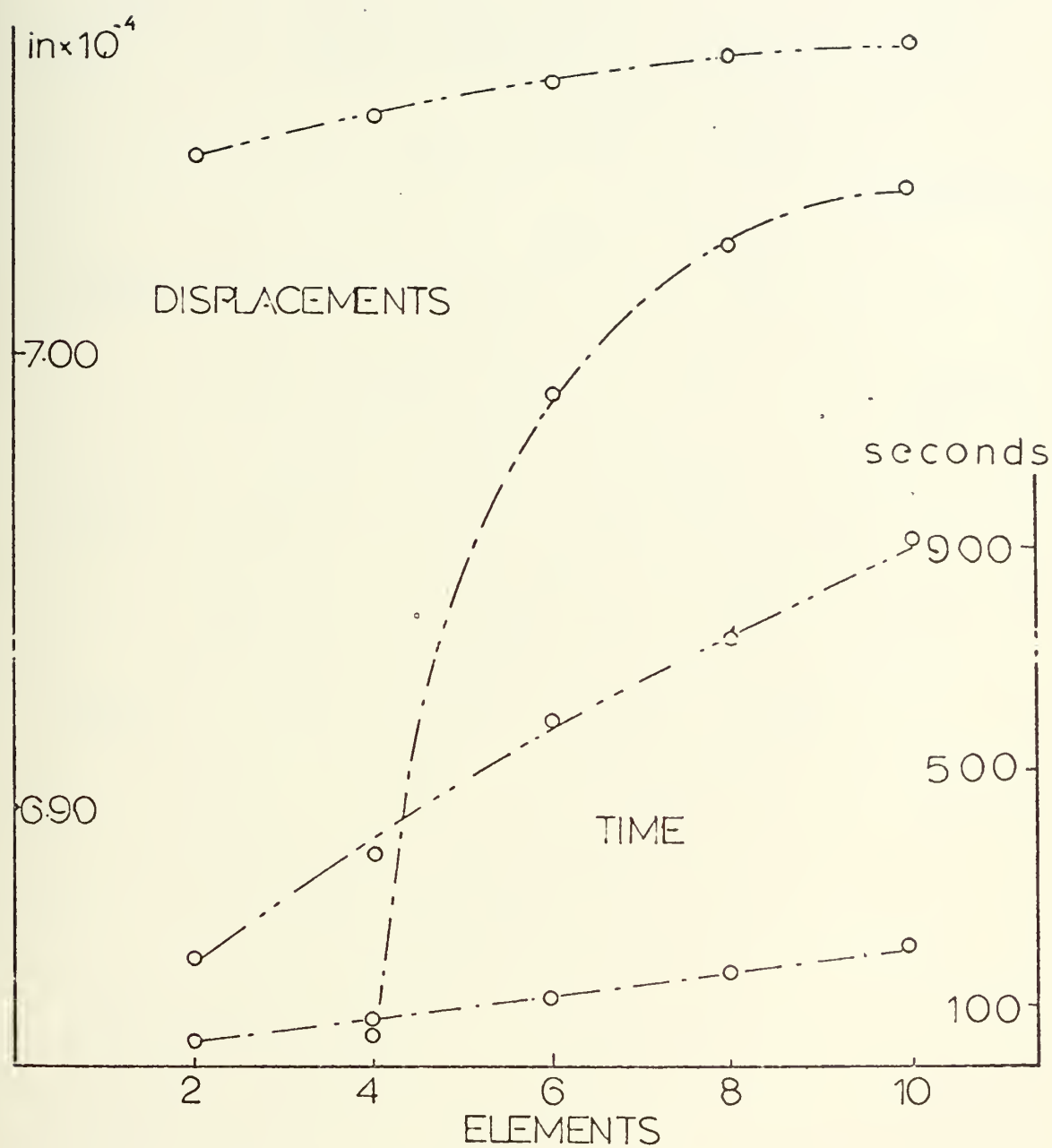
Cubic elements

Numbers represent proportions of total load applied on each node.

Figure 6







--- Cubic el. - CUB 4  
 - - - Quadr. el. - CUB 3

Figure 7



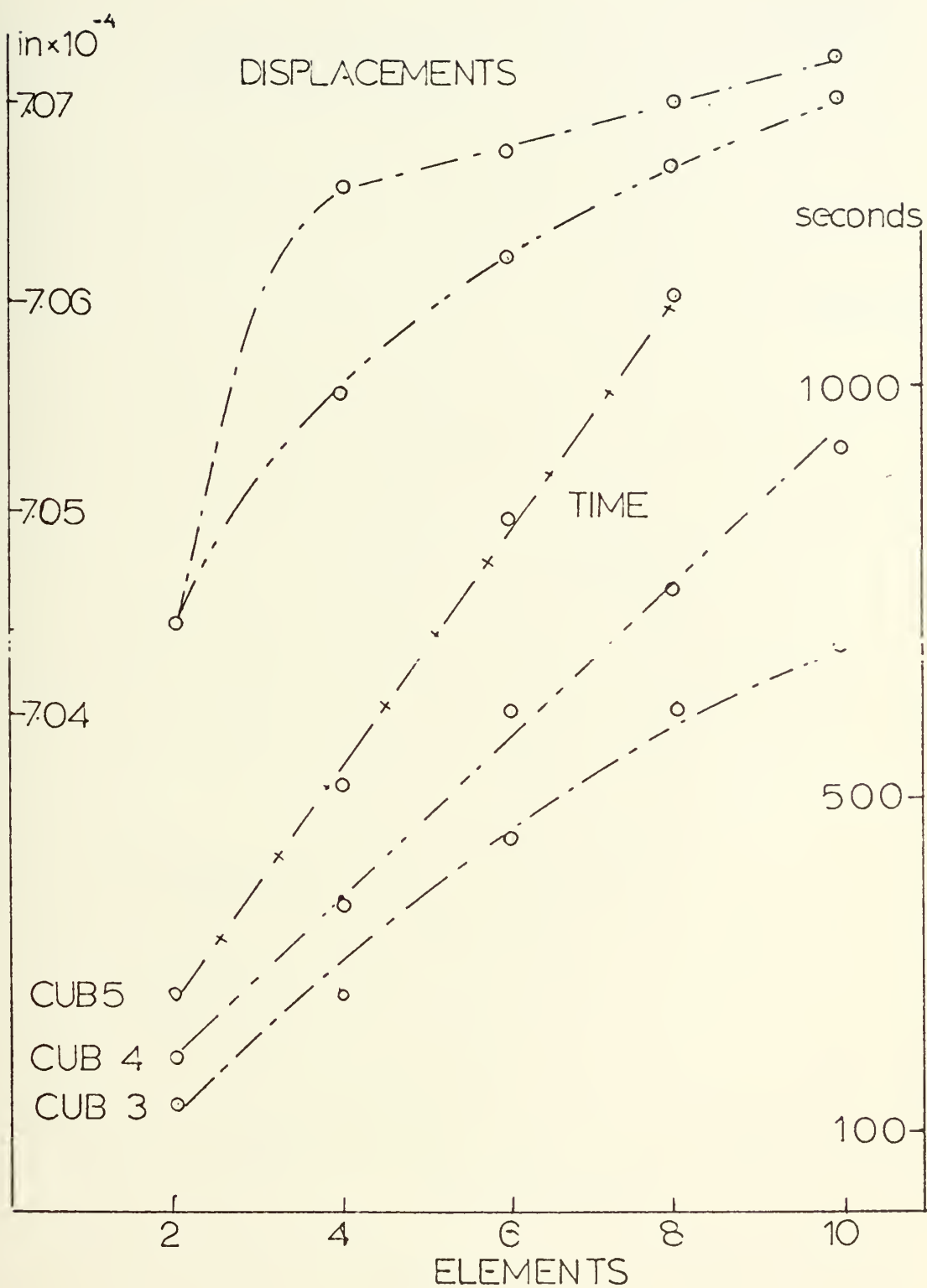


Figure 8



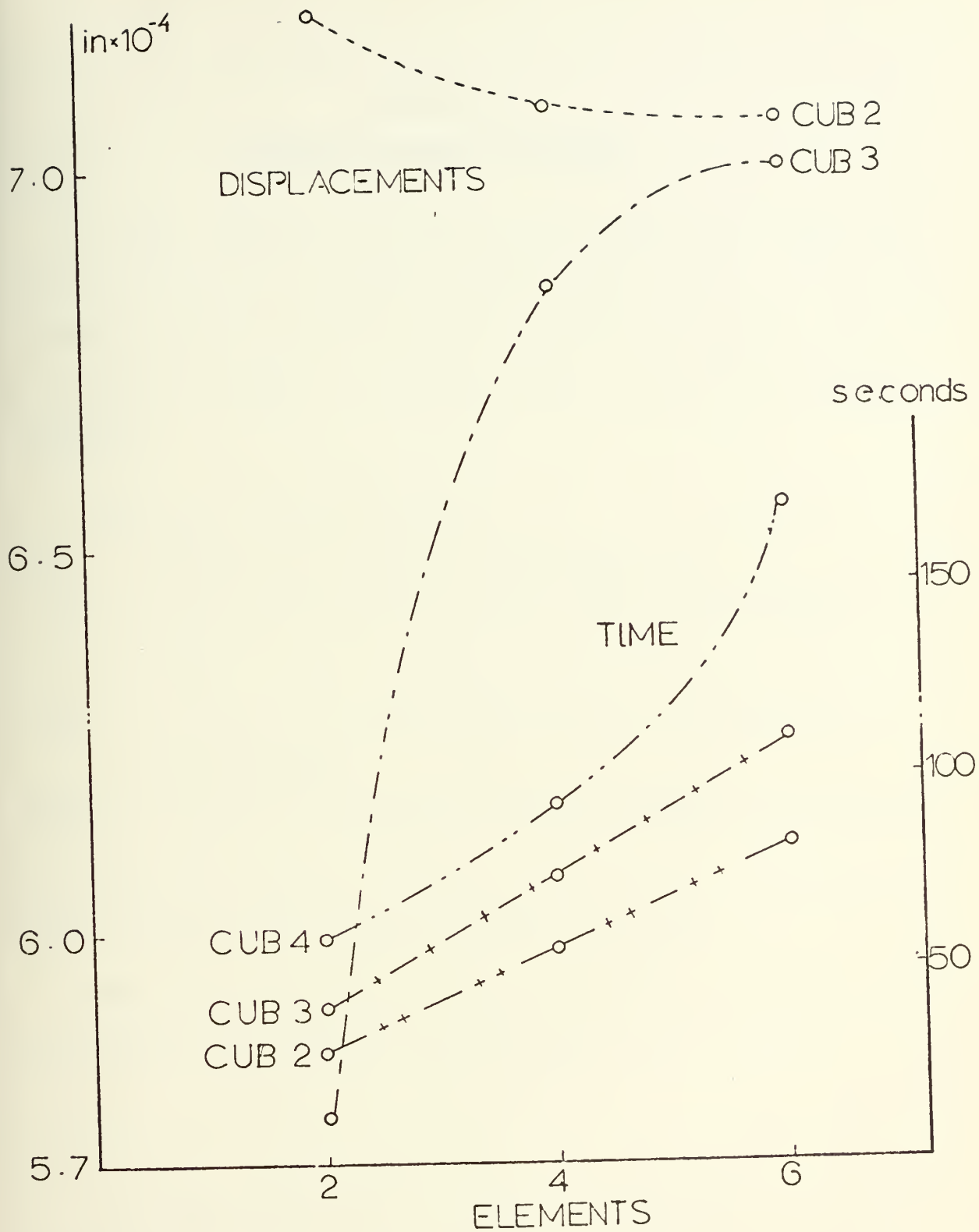


Figure 9



## APPENDIX A

### COMPUTER PROGRAM VARIABLES

In this Appendix the most important variables appearing in the programs are listed together with a brief description of their function. The variables of each group are sequenced in alphabetic order. Arrays are shown with the dimensions required.

#### A. DATA FILES CHARACTERISTICS

NRX : Number of records of file number "X"

LRBX : Length of records of file number "X" measured in bytes

LRWX: : Length of records of file number "X" measured in single words.

NREC7 : Number of records per total stiffness matrix block

NRX, LRBX AND LRWX are used only for checking the adequacy of disk storage space requested by the JCL cards.

NREC7 specified by the user, is used in subroutine DISK and controls the transfer of stiffness matrix blocks.

#### B. VARIABLES APPEARING IN 'COMMON' STATEMENTS

AK1(NS,NS) : Work space used primarily for storage of stiffness matrix blocks when transferred in core. Alternatively is used for storing





equivalenced arrays and thus reduce the space requirements. Similarly for AK2 and AK3.

- BCON(3) : Array of non zero predefined displacement components for one node. It is used only in entry BCOND2 of subroutine BCOND. The complete array of non zero predefined displacements is stored on disk and only the components for one node at a time are present in core.
- CLOAD(4) : Concentrated load data for one node. The first three elements represent the components of the load and the fourth the node number. It is used only in subroutine FLOAD. The complete array of concentrated load data is stored on disk and only data for one node at a time are present in core.
- COORD(3) : Array of global coordinates for one node. The complete array of nodal coordinates is stored on disk in node sequence following the numbering convention for the whole structure. Coordinates of only one node at a time are present in core.
- COREL(NPEL,3) : Array containing the global coordinates for all nodes of one element. These arrays are formed by SUBROUTINE SORT one at a time and subsequently stored on disk. Only one such array is present in core every time.



ELAST(616) : Matrix of elastic constants of elements formed by subroutine ELPROP.

ELDAT(10,3) : Array containing information on properties of materials composing the structure. It is used in ELPROP for the formation of ELAST.

KEL : Index number indicating whether printing of element nodal strains and stresses is required or not.

MM : Number of stiffness matrix blocks per row (see Ref. 2)

NBC(4) : Boundary condition information for one node. The first element represents the node number and the remaining the boundary condition codes. The complete set of such arrays is stored on disk in node number sequence and only one is present in core at a time.

NBCH : Highest node number at which a boundary condition is applied.

NBCL : Lowest node number at which a boundary condition is applied.

NCLD : Total number of nodes at which a concentrated load is applied.

NCON(NPEL) : Array of connectivities for one element. This array shows the correspondence of element node numbers in the two numbering systems employed, namely, element numbering system and entire structure numbering system. The values of



the elements of the array represent the node numbers in the structure numbering system and are sequenced according to the element numbering system. Thus the third element of the array represents node number three of the element and its value represents the number of this node in the structure numbering system. The complete set of these arrays is stored on disk and only one at a time is brought into core.

- NCON21 : Indicates the material of the corresponding element and is used in the case the structure is composed of more than one material. The values of this variable must conform with the material number given in array ELDAT.
- ncount : Number of coefficients per stiffness matrix block.
- NDF : Number of spacial degrees of freedom. In the present form of the program its value is constant and equal to three. However, it is used throughout the origin as a variable in order to provide for future developments of the programs as for example degeneration to two dimensional elements.
- NDFP : Constant equal to:  $NDF+1$
- NDPT : Total number of nodal points in the structure.
- NEL : Total number of finite elements used in the problem.



NEQ : Total number of equations of system (2.5).  
 NLINE : Number of lines to be printed per page in the  
 output for displacement, reactions, strains and  
 stresses.  
 NMAT : Number of materials of which the structure is  
 composed.  
 NN : Number of stiffness matrix blocks per column (See  
 Ref. 2).  
 NPBC : Total number of nodes at which boundary conditions  
 are applied.  
 NPBC2 : Total number of nodes at which non zero predefined  
 displacements are applied.  
 NPEL : Number of nodes per element.  
 NREC7 : Number of records per stiffness matrix block.  
 NS : Stiffness matrix block size.  
 NST : Size of element stiffness matrix.  
 NSTF : Number of coefficients per element stiffness matrix.  
 SSSS1(7) : Work vector used for transferring to and from data  
 files the algebraic sums of stress or strain  
 components contributed by elements sharing a common  
 node. The seventh element of the array is used to  
 count the number of occurrences of the node. Such  
 arrays are formed in subroutine STRESS and further  
 used in MAINE for the evaluation of average  
 strains and stresses.  
 UJNT(3) : Array of displacement components for one node. The  
 complete set of such arrays is stored on disk and





displacements for only one node at a time  
are present in core.

### C. OTHER IMPORTANT VARIABLES

In order to avoid redundant definitions, the most important variables of each subroutine that have not been presented in Sections A and B of this Appendix are explained below.

#### 1. Subroutine ELPROP

E : Young's modulus  
PR : Poisson's ratio

#### 2. Subroutine CUB

STK(NST,NST) : Element stiffness matrix  
X,Y,Z : Gauss points coordinates

#### 3. Subroutine FORMK

AJ(NDF,NDF) : Jacobian matrix  
AK(NST,NST) : Integrant of stiffness matrix integral  
formula  
B(6,NST) : Matrix  $[N^*] = [D][N]$   
CORDG(NPEL,NDF) : Element nodal coordinate array in the local  
system  
DNX(NDF,NPEL) : Array of derivatives of shape functions with  
respect to  $x,y,z$ .  
W1(NDF,NPEL) : Array of derivatives of shape functions with  
respect to  $\xi,\eta,\zeta$ .

#### 4. Subroutine BCOND

NBC : Stiffness matrix row number corresponding to  
the 'z' component of the boundary condition  
examined.



IBL,IBH : Lowest and highest row number in the block examined.

INDEX : Number of boundary conditions applied.

AKW1(NCOUNT) : Diagonal stiffness matrix block.  
AK1(NS,NS)

AKW2(NCOUNT) : Offdiagonal stiffness matrix block.  
AK2(NS,NS)

RB1(NS) : Load vector corresponding to row 'I'.

RB2(NS) : Load vector corresponding to blocks symmetric to the offdiagonal blocks of row 'I'.

MM1 : Specifies the extreme offdiagonal block to be examined in each row in order to avoid blank blocks.

IFR : Specifies whether any boundary conditions have to be applied to the diagonal block of row 'I' or not.

#### 5. Subroutine SOLVE

KMM : Same as MM1 in MERGE.

IFLG : Indicates whether block AK2 is singular or nearly singular.

#### 6. Subroutine DISP

JLL : Indicates the number of non blank elements in the last load vector

#### 7. Subroutine STRESS

CORDG(NPEL,3) : Vector of element nodal coordinates in the local system.

SNELM(NPEL,6) : Array of element nodal strains.

SSELM(NPEL,6) : Array of element nodal stresses.



SSNN(6) : Strain vector for one node.  
SSSS(6) : Stress vector for one node.  
UEL(NST) : Vector of element nodal displacements.



## APPENDIX B

### DIRECT ACCESS DATA FILES REQUIREMENTS

In this Appendix the direct access data file requirements as well as the necessary control cards for use of the compiler of release 18 of an IBM 360 operating system are described.

#### A. DATA FILE SPACE REQUIREMENTS

The data file space requirements are described by the variables NRX, LRBX, LRWX as they are explained in Section A of Appendix A.

The required values of these variables are as follows:  
(the values of LRWX are not presented since always  $LRWX = LRBX/4$ )

NR7	=	MM x NN x NREC7
LRB7	=	NCOUNT x 8/NREC7
NR8	=	NEL x 4 for quadratic elements NEL x 12 for cubic elements
LRB8	=	7200 for quadratic elements 6144 for cubic elements
NR9	=	NEL
LRB9	=	80 for quadratic elements 128 for cubic elements
NR10	=	NDPT
LRB10	=	24





NR12 = NDPT

LRB12 = 48

NR13 = NDPT

LRB13 = 48

NR14 = NCLD

LRB14 = 32

NR15 = NPBC

LRB15 = 16

NR16 = NEL

LRB16 = 4

NR17 = NPBC

LRB17 = 32

NR19 = NEL

LRB19 = 480

NR20 = NN

LRB20 = NS x 8

NR21 = NN x NS/NDF

LRB21 = 24

NR22 = NPBC2

LRB22 = 24

## B. CONTROL CARDS

Job control language cards for an IBM 360 operating system are of the form:

```
GO.FTXXF001 DD UNIT = SYSDA, SPACE = (LRBX, (NRX,1)),DISP=
      (NEW,DELETE)
```

where X and XX represent the number of the corresponding file.



These cards may be produced by the auxiliary computer program named JCL and whose listing is presented in Appendix H. This program is capable of printing and punching the JCL cards as well as the data files characteristics (Section IV.C.1) for a series of problems in one run. One data card is required for each problem. This card must contain the variables: NEL, NPEL, MM, NS, NDPT, NPBC, NPBC2, NCLD, NREC7, NMAT punched in format (1X,1015). Following the data card of the last problem, a card with a negative integer punched in columns one to six right justified, must be given. The control cards required for this program in order to be used in an IBM 360 OS are shown in Appendix F.

In case the value of MM is not known, it can be computed by the program from connectivity data. In this case MM is given the value zero in the data card described above and which must be immediately followed by the data cards:

1. Object Time Format (20A4)

Number of cards: one

The format under which connectivity data will be read is punched. This format must provide for reading NPEL integers at a time, representing one row of the connectivity matrix.

2. Connectivity Data

Number of cards: NEL x (number of cards per row of connectivity matrix as per specified format.)

Each individual row of the connectivity matrix is punched in sequence under the specified format.



The input data of program JCL are printed in the output for checking purposes. A sample output is presented in Appendix E.

### C. DEFINE FILE STATEMENTS

The DEFINE FILE statements contained in subroutine DISK and defining the space requirements for random access files must be adjusted to the needs of the larger problem of each run. The form of these statements is:

```
DEFINE FILE X(NRX,LRWX,U,IX)
```

where X represents the number of the corresponding file.

These statements are also produced by program JCL as described in Section B of this Appendix.



# APPENDIX C

## SAMPLE INPUT DATA DECKS FOR TRISOP

### A. QUADRATIC ELEMENTS

16	7200	1800	4	8	7200	1800
2	80	32	24	6	32	56
14	32	56	14	3	32	6
16	0	0	6	32	2	480
2	480	120	40	24	6	0
0						

START  
1 SIMPLY SUPPORTED BEAM - QUADRATIC ELEMENTS - MESH 1X1X2  
2 60  
3 1  
4 1  
5 1  
6 1  
7 1  
8 1  
9 1  
10 1  
11 1  
12 1  
13 1  
14 1  
15 1  
16 1  
17 1  
18 1  
19 1  
20 1  
21 1  
22 1  
23 1  
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16 -64:0
18 -16:0
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I



## APPENDIX D

### SAMPLE OUTPUT FOR TRISOP

A sample output for the solution of a problem by use of quadratic elements is presented in this Appendix. The problem chosen is that of a simply supported beam with a line load acting on the mid-section and discretized in a  $1 \times 1 \times 2$  mesh. This problem is one of those discussed in Section V.

The form of the output for a solution using cubic elements will be identical except for the connectivity matrix. The form of the connectivity matrix for the same problem solved by use of cubic elements will be as follows:

A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P
Q	R	S	T	U	V	W	Y	Z	A1	B1	C1	D1	E1	F1	G1
9	7	5	1	2	3	4	6	8	12	11	10	15	13	14	16
19	17	18	20	29	27	25	21	22	23	24	26	28	32	31	30
29	27	25	21	22	23	24	26	28	32	31	30	35	33	34	36
39	37	38	40	49	47	45	41	42	43	44	46	48	52	51	50





## SIMPLY SUPPORTED BEAM - QUADRATIC ELEMENTS - MESH 1X1X2

NEL...TOTAL NUMBER OF ELEMENTS..... 2  
 NODT...TOTAL NUMBER OF NODAL POINTS..... 32  
 NMAT...NUMBER OF DIFFERENT MATERIALS..... 1  
 (MAXIMUM IS 10)  
 NS....BLOCK SIZE FOR THE LARGE CAPACITY SOLVER..... 60  
 NPROC...NUMBER OF NODAL POINTS WITH ROUNO. COND..... 6  
 NPROC?...NUMBER OF NODAL POINTS WITH PREDEF. DISPL..... 0  
 NCLO...NUMBER OF NODAL POINTS WITH CONC. LOAD..... 3

A B C D E F G H I J K L M N O P Q R S T CONN. MTR.

6 4 1 2 3 5 8 7 11 9 10 12 18 16 13 14 15 17 20 19  
 18 16 13 14 15 17 20 19 23 21 22 24 30 28 25 26 27 29 32 31

NEW...TOTAL NUMBER OF EQUATIONS..... 96  
 NBAND...HALF-BAND WIDTH OF THE SYSTEM..... 60  
 NN....NUMBER OF BLOCKS PER COLUMN..... 2  
 MM....NUMBER OF BLOCKS PER ROW..... 2  
 NCOUNT...NUMBER OF COEFFICIENTS PER BLOCK..... 3600  
 NSTF...NUMBER OF COEFFICIENTS PER ELEMENT..... 3600

COORD.	JOINT	X	Y	Z	KND
	1	0.0	2.00000	24.00000	0
	2	0.0	1.00000	24.00000	0
	3	0.0	0.0	24.00000	0
	4	1.00000	2.00000	24.00000	0
	5	1.00000	0.0	24.00000	0
	6	2.00000	2.00000	24.00000	0
	7	2.00000	1.00000	24.00000	0
	8	2.00000	0.0	24.00000	0
	9	0.0	2.00000	18.00000	0
	10	0.0	0.0	18.00000	0
	11	2.00000	2.00000	18.00000	0
	12	2.00000	0.0	18.00000	0
	13	0.0	2.00000	12.00000	0
	14	0.0	1.00000	12.00000	0
	15	0.0	0.0	12.00000	0
	16	1.00000	2.00000	12.00000	0
	17	1.00000	0.0	12.00000	0
	18	2.00000	2.00000	12.00000	0
	19	2.00000	1.00000	12.00000	0
	20	2.00000	0.0	12.00000	0
	21	0.0	2.00000	6.00000	0
	22	0.0	0.0	6.00000	0
	23	2.00000	2.00000	6.00000	0
	24	2.00000	0.0	6.00000	0
	25	0.0	2.00000	0.0	0
	26	0.0	1.00000	0.0	0
	27	0.0	0.0	0.0	0
	28	1.00000	2.00000	0.0	0
	29	1.00000	0.0	0.0	0
	30	2.00000	2.00000	0.0	0
	31	2.00000	1.00000	0.0	0
	32	2.00000	0.0	0.0	0

NMAT	Y.MOD.	P.RATIO	ALPHA	ELEMENT DATA
1	30000000.	0.30000	0.0	



N.PT	XLD	YLD	ZLD	CONCENTRATED LOAD DATA
13	0.0	-16.00000	0.0	
16	0.0	-64.00000	0.0	
18	0.0	-16.00000	0.0	
J1				BOUNDARY CONDITIONS

	X	Y	Z
3	1	1	0
5	1	1	0
6	1	1	0
27	1	1	1
28	1	1	1
22	1	1	1

INPUT \*\*\*\*\*  
TIME = 14.62 SECONDS(CPU = 0.80 SECONDS)

\*\*\*\*\* SORT \*\*\*\*\*  
TIME = 5.86 SECONDS(CPU = 0.26 SECONDS)

\*\*\*\*\* FSTF \*\*\*\*\*  
TIME = 27.50 SECONDS(CPU = 15.61 SECONDS)

\*\*\*\*\* MERGE \*\*\*\*\*  
TIME = 17.20 SECONDS(CPU = 3.21 SECONDS)

\*\*\*\*\* FLQAD \*\*\*\*\*  
TIME = 5.50 SECONDS(CPU = 0.04 SECONDS)

\*\*\*\*\* BCOND \*\*\*\*\*  
TIME = 4.63 SECONDS(CPU = 1.10 SECONDS)

CONDITION NUMBER	2.4258750813639340D 04	2.8080341840341820D 09	8.635051090265440D-06
CONDITION NUMBER	2.4372551372157370D 04	1.0013305793152500D 09	2.4339946033916030D-05
***** SOLVE *****			
TIME = 39.82 SECONDS(CPU = 27.11 SECONDS)			



D-I-S-P-L-A-C-F-M-E-N-T-S

N.P.T

X

Y

Z

1	-1.61088015031574100-06	-1.55090062460768700-06	3.44636096173503600-07
2	-2.2712617666770400-07	-1.74041306315188200-06	8.62210268881764800-05
3	0.0	0.0	1.72165169132805000-04
4	0.0	-6.004330955091583700-07	1.71719568077456800-07
5	0.0	0.0	1.72335510347767100-04
6	1.61088015030614100-06	-1.50909062460234900-06	3.44636096144788500-07
7	2.2712617662963600-07	-1.74041306314681300-06	8.6210668817408700-05
8	0.0	0.0	1.72165169132775400-04
9	-2.20441064927391000-06	-4.1768449203517700-04	3.75608945736575300-05
10	-2.17340379716154100-06	-4.17309287436146700-04	1.35013780892295700-04
11	-2.20441064910861700-06	-4.17268449203485100-04	3.75608945729458600-05
12	-2.17340379725621000-06	-4.17309287436117000-04	1.35013780892283700-04
13	-2.74416749064152200-06	-5.76053690506871500-04	8.61256085980453800-05
14	-3.14305456129398400-07	-5.76050637049747700-04	3.6143592940701100-05
15	-2.69693397579002400-06	-5.75584438974499500-04	8.6143613100717902800-05
16	-1.55660016046181600-16	-5.76050637049747700-04	3.6143613100717902800-05
17	-2.71800539854986000-17	-5.76050637049747700-04	8.6143613100717902800-05
18	-2.74416749040876900-06	-5.76050637049747700-04	3.6143613100717902800-05
19	-3.14305455986186900-07	-5.76050637049747700-04	8.6143613100717902800-05
20	-2.69693397584419700-06	-5.76050637049747700-04	3.6143613100717902800-05
21	-2.20441064910861700-06	-4.17268449203485100-04	1.35013780892283700-04
22	-2.17340379725621000-06	-4.17309287436117000-04	3.75608945729458600-05
23	-2.74416749064152200-06	-5.76050637049747700-04	8.61256085980453800-05
24	-3.14305456129398400-07	-5.76050637049747700-04	3.6143592940701100-05
25	-2.69693397579002400-06	-5.75584438974499500-04	8.6143613100717902800-05
26	-1.55660016046181600-16	-5.76050637049747700-04	3.6143613100717902800-05
27	-2.71800539854986000-17	-5.76050637049747700-04	8.6143613100717902800-05
28	-2.74416749040876900-06	-5.76050637049747700-04	3.6143613100717902800-05
29	-3.14305455986186900-07	-5.76050637049747700-04	8.6143613100717902800-05
30	-2.69693397584419700-06	-5.76050637049747700-04	3.6143613100717902800-05
31	-2.20441064910861700-06	-4.17268449203485100-04	1.35013780892283700-04
32	-2.17340379725621000-06	-4.17309287436117000-04	3.75608945729458600-05



# R-E-A-C-T-I-O-N-S

N.PT	X	Y	Z
3	2.3658853372884870D 01	-1.509197375734090D 01	0.0
5	2.6762408448878810D-11	-1.7816052489112260D 01	0.0
8	-2.5658853372945610D 01	-1.509197375734090D 01	0.0
27	2.4127265073037160D 01	-1.4815110525534580D 01	4.5190459456394300D 00
29	1.4322801094607820D-10	-1.836977824957750D 01	-9.0380918915346030D 00
32	-2.4127365073164580D 01	-1.4815110525420530D 01	4.5190459457794850D 00

## EQUILIBRIUM CHECK

-1.8189894035459560D-11 -9.6000000001027870D 01 -1.1568812574580530D-10  
 \*\*\* DISP \*\*\*  
 TIME = 11.36 SECONDS (CPU = 0.92 SECONDS)





JOINT	SNX/SSX	SNY/SSY	SNZ/SSZ	SNXY/SSXY	SNYZ/SSYZ	SNZX/SSZX
6	1.610880D-06	1.154475D-06	-5.257005D-06	7.093886D-09	4.875068D-06	4.243017D-08
6	-5.950550D-00	-1.048267D-01	-1.244440D-02	8.185253D-01	5.625078D-01	4.895789D-01
4	1.610880D-06	1.627256D-06	-5.264582D-06	-9.330200D-19	5.4724016D-06	2.175485D-18
4	2.101040D-00	2.478944D-00	-1.555635D-02	-1.076562D-11	5.450788D-01	2.510175D-11
1	1.610880D-06	1.154475D-06	-5.257005D-06	-7.093886D-08	4.875068D-06	-4.243017D-08
1	-5.950550D-00	-1.048267D-01	-1.244440D-02	-9.185253D-01	5.625078D-01	-4.895789D-01
2	2.271296D-07	-7.754503D-07	-2.379592D-08	1.401210D-07	5.029041D-06	-2.831732D-08
2	-4.660566D-00	-2.779703D-01	-1.045116D-01	1.616781D-00	5.802739D-01	-3.267383D-01
3	-2.365985D-19	-2.705376D-06	5.216808D-06	3.511808D-07	5.138995D-06	-1.590393D-07
3	4.346709D-01	-1.896466D-01	1.638550D-02	4.052087D-00	5.929610D-01	-1.835069D-00
5	-2.365885D-19	-2.232595D-06	5.234521D-06	-1.605445D-18	4.869703D-06	-1.253741D-18
5	5.195641D-01	4.349773D-01	1.727530D-02	-1.552437D-11	5.618888D-01	-1.446624D-11
8	-2.365885D-19	-2.705376D-06	5.216808D-06	-3.511808D-07	5.138995D-06	1.590393D-07
8	4.346709D-01	-1.896466D-01	1.638550D-02	-4.052087D-00	5.929610D-01	1.835069D-00
7	2.271296D-07	-7.754503D-07	-2.379592D-08	-1.401210D-07	5.029041D-06	3.831732D-08
7	-4.660566D-00	-2.779703D-01	-1.045116D-01	1.616781D-00	5.802739D-01	3.267383D-01
11	2.204411D-06	2.214459D-06	-7.148414D-06	2.323242D-07	-8.066939D-07	-3.953355D-08
11	3.628898D-00	3.860787D-00	-2.122055D-02	2.680663D-00	-9.308006D-00	-4.561563D-01
9	2.204411D-06	2.214459D-06	-7.148414D-06	-2.323242D-07	-8.066939D-07	3.953355D-08
9	3.628898D-00	3.860787D-00	-2.122055D-02	-2.680663D-00	-9.308006D-00	4.561563D-01
10	-2.173404D-06	-2.173611D-06	7.166988D-06	-1.293326D-07	-8.051734D-07	-9.662199D-08
10	-1.348238D-00	-1.352019D-00	2.141983D-02	-1.492299D-00	-9.290462D-00	-1.114869D-00
12	-2.173404D-06	-2.173611D-06	7.166988D-06	1.293326D-07	-8.051734D-07	9.662199D-08
12	-1.348238D-00	-1.352019D-00	2.141983D-02	1.492299D-00	-9.290462D-00	1.114869D-00
18	2.744167D-06	2.221019D-06	-9.039824D-06	-4.581042D-07	5.036313D-06	-1.214973D-07
18	-7.195626D-00	-1.926929D-01	-2.791339D-02	-5.285817D-00	5.811131D-01	-1.401891D-00
16	2.744167D-06	1.984354D-06	-9.047401D-06	4.587767D-18	5.037004D-06	5.932619D-18
16	-1.142289D-01	-2.895734D-01	-2.835360D-02	5.305139D-11	5.811928D-01	6.845329D-11
13	2.744167D-06	2.221019D-06	-9.039824D-06	4.581042D-07	5.036313D-06	1.214973D-07
13	-7.195626D-00	-1.926929D-01	-2.791339D-02	-5.285817D-00	5.811131D-01	1.401891D-00
14	3.143055D-07	-2.371258D-07	3.497525D-08	3.500359D-07	4.927879D-06	1.160637D-07
14	9.194346D-00	-3.530990D-00	2.748264D-00	4.038876D-00	5.686015D-01	1.339197D-00
15	-2.696954D-06	-2.695270D-06	9.117169D-06	2.419676D-07	4.775427D-06	-3.420464D-08
15	2.232791D-00	2.271643D-00	2.748664D-02	2.791934D-00	5.510108D-01	-3.946689D-01
17	-2.696954D-06	-2.931935D-06	9.134882D-06	1.017617D-18	4.657877D-06	-5.072033D-18
17	-1.556752D-00	-6.979390D-00	2.714956D-02	1.174174D-11	5.374473D-01	-5.852346D-11
20	-2.696954D-06	-2.695270D-06	9.117169D-06	-2.419676D-07	4.775427D-06	3.420464D-08
20	2.232791D-00	2.271643D-00	2.748664D-02	-2.791934D-00	5.510108D-01	3.946689D-01
19	3.143055D-07	-2.371258D-07	3.497525D-08	-3.500359D-07	4.927879D-06	-1.160637D-07
19	9.194346D-00	-3.530990D-00	2.748264D-00	-4.038876D-00	5.686015D-01	-1.339197D-00



JOINT	SNX/SSX	SNY/SSY	SNZ/SSZ	SNXY/SSXY	SNYZ/SSYZ	SNZX/SSZX
18	2.7441670-06	2.2210190-06	-9.0459160-06	-4.5810420-07	-5.0745380-06	4.5522140-08
18	-7.2958840 00	-1.9368540 01	-2.7036730 02	-5.2859170 00	-5.8573640 01	5.7152470-01
16	2.7441670-06	1.98433540-06	-9.0441370-06	4.5977870-18	-5.0165180-06	-5.7259030-18
16	-1.1366400 01	-2.8900550 01	-2.8346420 02	5.33051390-11	-5.7887510 01	-6.6968110-11
13	2.7441670-06	2.2210190-06	-9.0459160-06	4.5810420-07	-5.0745380-06	-4.9532140-08
13	-7.2958840 00	-1.9368540 01	-2.7036730 02	5.2859170 00	-5.8573640 01	-5.7152470-01
14	3.1430550-07	-2.3712580-07	3.8500930-08	3.5003590-07	-4.9677170-06	-1.6375280-07
14	9.2553670 00	-3.4699680 00	2.8006480 00	4.3388760 00	-5.7319810 01	-1.8845560 00
15	-2.6069540-06	-2.6952700-06	9.1290130-06	2.44196760-07	-4.8150930-06	-1.3234860-07
15	2.4533640 00	2.4922160 00	2.7538110 02	2.7319340 00	-5.5558760 01	-1.5270990 00
17	-2.6069540-06	-2.9319350-06	9.1290130-06	1.0170170-18	-4.63732160-06	1.8151920-18
17	-1.7036430 00	-7.1262810 00	2.7114290 02	1.1741740-11	-5.3507490 01	2.0944520-11
20	-2.6069540-06	-2.8952700-06	9.1290130-06	-2.44196760-07	-4.8150930-06	1.3234860-07
20	2.4533640 00	2.4922160 00	2.7538110 02	-2.7319340 00	-5.5558760 01	1.5270990 00
19	3.1430550-07	-2.3712580-07	3.8500930-08	-3.5003590-07	-4.9677170-06	1.6375280-07
19	9.2553670 00	-3.4699680 00	2.8006480 00	-4.3388760 00	-5.7319810 01	1.8894560 00
23	2.2048360-06	2.2139360-06	-7.1541100-06	2.3294500-07	8.2700630-07	7.5957620-08
23	3.5384180 00	3.7484190 00	-2.1243730 02	2.6878270 00	9.5423810 00	8.7643410-01
21	2.2048360-06	2.2139360-06	-7.1541100-06	-2.3294500-07	8.2700630-07	-7.5957620-08
21	3.5384180 00	3.7484190 00	-2.1243730 02	-2.6878270 00	9.5423810 00	-8.7643410-01
22	-2.1664590-06	-2.1748440-06	7.1801090-06	-1.2133200-07	8.2543810-07	1.8252910-07
22	-8.6201820-01	-1.0555100 00	2.1482800 02	-1.3999850 00	9.5242850 00	2.1061050 00
24	-2.1664590-06	-2.1748440-06	7.1801090-06	1.2133200-07	8.2543810-07	-1.8252910-07
24	-8.6201820-01	-1.0555100 00	2.1482800 02	1.3999850 00	9.5242850 00	-2.1061050 00
30	1.6134530-06	1.1556670-06	-5.2625040-06	7.9986750-08	-4.7935350-06	1.0238310-07
30	-5.9229360 00	-1.6487230 01	-1.6460120 02	9.2292410-01	-5.5310020 01	1.1813430 00
28	1.6134530-06	1.6288770-06	-5.2611250-06	1.0335010-18	-4.7640580-06	-2.8392540-18
28	2.2928470 00	2.6487980 00	-1.5635120 02	1.2501930-11	-5.4969860 01	-3.2760630-11
25	1.6134530-06	1.1556670-06	-5.2625040-06	-7.9986750-08	-4.7935350-06	-1.0238310-07
25	-5.9229360 00	-1.6487230 01	-1.6460120 02	-9.2292410-01	-5.5310020 01	-1.1813430 00
26	2.2367530-07	-7.7496780-07	-1.0796660-06	1.3969450-07	-4.5477230-06	1.2469940-07
26	-4.7225000 00	-2.7758110 01	-1.0341080 01	1.6118600 00	-5.7089170 01	1.4289390 00
27	-2.4127370-19	-2.7056020-06	5.2303050-06	3.5937580-07	-5.0579610-06	4.9740680-07
27	4.3696780 01	-1.8740200 01	1.6439910 02	4.1466440 00	-5.8361090 01	5.7393100 00
29	-2.4127370-19	-2.2323920-06	5.2267870-06	-7.0698130-19	-4.9101680-06	-2.4219320-18
29	5.1826070 01	3.0933250-01	1.7244420 02	-8.1574770-12	-5.66555780 01	-2.7945370-11
32	-2.4127370-19	-2.7056020-06	5.2303050-06	-2.5937580-07	-5.0579610-06	-4.9740680-07
32	4.3696780 01	-1.8740200 01	1.6439910 02	-4.1466440 00	-5.8361090 01	-5.7393100 00
31	2.2367530-07	-7.7496780-07	-1.0796660-06	-1.3969450-07	-4.5477230-06	-1.2469940-07
31	-4.7225000 00	-2.7758110 01	-1.0341080 01	-1.6118600 00	-5.7089170 01	-1.4289390 00



A-V-E-R-A-C-E S-T-R-E-S-S-E-S AT THE JOINTS									
JCINT	SSX	SSY	SSZ	SSXY	SSYZ	SSZY	SSZX		
1	-5.9505500	00	1.6432970	01	1.6440200	02	3.1852530	01	5.9020780
2	-4.6605660	00	1.0451160	00	0.1620870	00	-1.6157280	00	5.9020780
3	-4.3467000	01	1.0331500	02	1.0765620	01	-1.40765620	01	5.9020780
4	-2.1010460	00	1.0331500	02	1.0765620	01	-1.40765620	01	5.9020780
5	-5.1956410	01	1.7244020	02	1.6440200	01	-1.85524370	01	5.9020780
6	-4.1950550	00	1.6432970	01	1.6440200	00	-1.6157280	00	5.9020780
7	-4.6605660	00	1.0451160	00	0.1620870	00	-1.40765620	00	5.9020780
8	-4.3467000	01	1.0331500	02	1.0765620	01	-1.40765620	01	5.9020780
9	-3.6288980	01	1.8964660	01	1.2209670	00	-2.6922990	00	5.9020780
10	-1.3482380	00	1.3530780	00	1.2209670	00	-2.6922990	00	5.9020780
11	-3.6288980	00	1.8964660	01	1.2209670	00	-2.6922990	00	5.9020780
12	-1.3482380	00	1.3530780	00	1.2209670	00	-2.6922990	00	5.9020780
13	-1.7244020	00	1.9318420	01	1.4250800	00	-5.28858170	00	5.9020780
14	-9.2248570	00	1.9318420	01	1.4250800	00	-5.28858170	00	5.9020780
15	-2.3430780	00	3.5004790	00	2.7919340	00	4.0388760	00	5.9020780
16	-1.3946400	01	2.3819300	00	3.0513390	01	5.17451740	01	5.9020780
17	-1.6301980	00	2.7919340	00	3.0513390	01	5.17451740	01	5.9020780
18	-7.2248570	00	2.7919340	00	3.0513390	01	5.17451740	01	5.9020780
19	-2.3430780	00	3.5004790	00	2.7919340	00	4.0388760	00	5.9020780
20	-3.5384180	00	3.7484190	00	3.9998500	00	-2.6922990	00	5.9020780
21	-8.53201820	01	1.0555100	00	1.4250800	00	-2.6922990	00	5.9020780
22	-3.5384180	00	3.7484190	00	3.9998500	00	-2.6922990	00	5.9020780
23	-5.9229260	00	1.0555100	00	1.4250800	00	-2.6922990	00	5.9020780
24	-5.9229260	00	1.0555100	00	1.4250800	00	-2.6922990	00	5.9020780
25	-4.7225000	00	2.7768110	01	2.6460120	02	-1.6113640	01	5.9020780
26	-4.3696780	01	1.8740200	01	1.0441080	02	1.40765620	00	5.9020780
27	-2.2928470	00	2.6497980	00	1.0441080	02	1.40765620	00	5.9020780
28	-5.1922930	01	3.0933320	00	1.5635120	02	-1.2501930	01	5.9020780
29	-4.7225000	00	2.7768110	01	2.6460120	02	-1.6113640	01	5.9020780
30	-4.7225000	00	2.7768110	01	2.6460120	02	-1.6113640	01	5.9020780
31	-4.3696780	01	1.8740200	01	1.0441080	02	1.40765620	00	5.9020780
32	-4.3696780	01	1.8740200	01	1.0441080	02	1.40765620	00	5.9020780





A-V-E-R-A-G-E S-T-R-A-I-N-S AT THE JOINT

JOINT	SNX	SNY	CNZ	SNXY	SNZY	SNZX
1	1.610980D-06	1.154475D-06	-5.257305D-08	-7.023886D-08	4.875068D-06	-4.243317D-08
2	2.256585D-19	-2.705376D-06	-5.216303D-06	1.291210D-07	5.029041D-06	-2.931732D-08
3	1.610880D-19	1.627259D-06	-5.264552D-06	3.351190D-07	5.138995D-06	-1.593039D-07
4	2.265985D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	-2.173485D-18
5	1.610880D-06	1.154475D-06	-5.257305D-08	-7.023886D-08	4.875068D-06	-4.243317D-08
6	2.271296D-07	-2.705376D-06	-5.216303D-06	1.291210D-07	5.029041D-06	2.831732D-08
7	2.265985D-19	1.627259D-06	-5.264552D-06	3.351190D-07	5.138995D-06	1.593039D-07
8	2.204411D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	3.653355D-08
9	2.273404D-06	2.173611D-06	-7.168980D-06	-1.233226D-07	8.051734D-07	-3.693333D-08
10	2.204411D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	-3.653355D-08
11	2.273404D-06	2.173611D-06	-7.168980D-06	-1.233226D-07	8.051734D-07	-3.693333D-08
12	2.273404D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	-3.653355D-08
13	3.1143055D-07	-2.371259D-07	3.673499D-08	4.560359D-07	-1.991877D-08	3.598256D-08
14	2.2696954D-06	-2.695270D-06	9.123541D-06	2.419676D-07	-1.933278D-08	-3.327661D-08
15	2.2744167D-06	1.984354D-06	9.045769D-06	4.597787D-07	1.004313D-08	-1.033579D-19
16	2.2696954D-06	-2.695270D-06	9.123541D-06	2.419676D-07	-1.933278D-08	3.598256D-08
17	2.2744167D-06	1.984354D-06	9.045769D-06	4.597787D-07	1.004313D-08	-1.033579D-19
18	3.1143055D-07	-2.371259D-07	3.673499D-08	4.560359D-07	-1.991877D-08	3.598256D-08
19	2.2696954D-06	-2.695270D-06	9.123541D-06	2.419676D-07	-1.933278D-08	-3.327661D-08
20	2.2744167D-06	1.984354D-06	9.045769D-06	4.597787D-07	1.004313D-08	-1.033579D-19
21	2.2696954D-06	-2.695270D-06	9.123541D-06	2.419676D-07	-1.933278D-08	3.598256D-08
22	2.2744167D-06	1.984354D-06	9.045769D-06	4.597787D-07	1.004313D-08	-1.033579D-19
23	2.204411D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	3.653355D-08
24	2.273404D-06	2.173611D-06	-7.168980D-06	-1.233226D-07	8.051734D-07	-3.693333D-08
25	2.204411D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	-3.653355D-08
26	2.273404D-06	2.173611D-06	-7.168980D-06	-1.233226D-07	8.051734D-07	-3.693333D-08
27	2.204411D-06	-2.232595D-06	-5.224521D-06	-6.330202D-07	4.469703D-06	-3.653355D-08
28	1.613453D-06	1.628877D-06	-5.261178D-06	3.593758D-07	-4.764951D-06	4.974680D-07
29	2.412737D-19	-2.232392D-06	-5.226787D-06	-7.069813D-19	-4.791016D-06	-2.832540D-18
30	1.613453D-06	1.155667D-06	-5.255694D-06	-7.069813D-19	-4.791016D-06	1.023932D-19
31	2.2367753D-07	-2.705678D-07	-1.979664D-08	-1.356945D-07	-4.947728D-06	-1.247460D-07
32	2.2367753D-07	-2.705678D-07	-1.979664D-08	-1.356945D-07	-4.947728D-06	-1.247460D-07

\*\*\*  
TIME = 32.78 SECONDS(CPU = 3.05 SECONDS)  
STRESS =

END OF PROBLEM





## APPENDIX E

### SAMPLE OUTPUTS FOR AUXILIARY PROGRAM JCL

Two different sample outputs for the auxiliary program JCL are presented in this Appendix, one for a problem utilizing quadratic elements and one for a problem utilizing cubic elements. Both problems are from those presented in Section V for a simply supported beam.



\*\*\*\*\* INPUT DATA \*\*\*\*\*

```
NEL....TOTAL NUMBER OF ELEMENTS.....4
NOPT...TOTAL NUMBER OF NODAL POINTS.....56
NMAT...NUMBER OF DIFFERENT MATERIALS.....1
        (MAXIMUM IS 10)
NS....BLOCK SIZE FOR THE LARGE CAPACITY SOLVER.....60
NPPC...NUMBER OF NODAL POINTS WITH BOUND. COND.....6
NPBC2...NUMBER OF NODAL POINTS WITH PREDEF. DISPL.....0
NCLO...NUMBER OF NODAL POINTS WITH CONC. LOAD.....2
NN....NUMBER OF BLOCKS PER COLUMN.....3
MM....NUMBER OF BLOCKS PER ROW.....2
NPCL...NUMBER OF NODES PER ELEMENT.....20
NREC7...NUMBER OF RECORDS OF FILE NO 7.....4
```

\*\*\*\*\* JCL CARDS \*\*\*\*\*

```
//GO.FT07F001 DD UNIT=SYSDA,SPACE=(7200,(0024,1)),DISP=(NEW,DELETE)
//GO.FT08F001 DD UNIT=SYSDA,SPACE=(7200,(0016,1)),DISP=(NEW,DELETE)
//GO.FT09F001 DD UNIT=SYSDA,SPACE=(0080,(0004,1)),DISP=(NEW,DELETE)
//GO.FT10F001 DD UNIT=SYSDA,SPACE=(0024,(0056,1)),DISP=(NEW,DELETE)
//GO.FT12F001 DD UNIT=SYSDA,SPACE=(0056,(0056,1)),DISP=(NEW,DELETE)
//GO.FT13F001 DD UNIT=SYSDA,SPACE=(0056,(0056,1)),DISP=(NEW,DELETE)
//GO.FT20F001 DD UNIT=SYSDA,SPACE=(0480,(0008,1)),DISP=(NEW,DELETE)
//GO.FT21F001 DD UNIT=SYSDA,SPACE=(0024,(0060,1)),DISP=(NEW,DELETE)
//GO.FT14F001 DD UNIT=SYSDA,SPACE=(0032,(0002,1)),DISP=(NEW,DELETE)
//GO.FT15F001 DD UNIT=SYSDA,SPACE=(0016,(0006,1)),DISP=(NEW,DELETE)
//GO.FT17F001 DD UNIT=SYSDA,SPACE=(0032,(0006,1)),DISP=(NEW,DELETE)
//GO.FT19F001 DD UNIT=SYSDA,SPACE=(0480,(0004,1)),DISP=(NEW,DELETE)
```

\*\*\*\*\* DEFINE FILE STATEMENTS \*\*\*\*\*

```
DEFINE FILE 7( 24,1800,U,17),8( 16,1800,U,13),          DSK00150
1             10( 56, 6,U,110),                      DSK00160
2             12( 56, 14,U,112),13( 56, 14,U,113),      DSK00170
3             20( 3, 120,U,120),21( 60, 6,U,121)        DSK00180
```

\*\*\*\*\* GENERAL FILE DATA \*\*\*\*\*

NRX = NUMBER OF RECORDS OF FILE X  
 LPBX = LENGTH OF RECORD OF FILE X IN BYTES  
 LRWX = -||- -||- -||- WORDS

NR7 = 24	NR8 = 16	NR9 = 4	NR10 = 56	NR12 = 56
LR7 = 7200	LR8 = 7200	LR9 = 80	LR10 = 24	LR12 = 56
LPW7 = 1800	LPW8 = 1800		LRW10 = 6	LRW12 = 14
NREC7 = 4				
NR13 = 56	NR14 = 3	NR15 = 6	NR16 = 0	NR17 = 6
LR13 = 56	LR14 = 32	LR15 = 16	LR16 = 0	LR17 = 32
LPW13 = 14				
NR19 = 4	NR20 = 3	NR21 = 60	NR22 = 0	
LR19 = 480	LR20 = 480	LR21 = 24	LR22 = 0	
	LRW20 = 120	LRW21 = 6		

NOTE : IF DATA OF FILES NO 16 OR 22 ARE ZERO THESE FILES ARE NOT REQUIRED



\*\*\*\*\* INPUT DATA \*\*\*\*\*

```
NEL....TOTAL NUMBER OF ELEMENTS.....8
NDPT...TOTAL NUMBER OF NODAL POINTS.....172
NMAT...NUMBER OF DIFFERENT MATERIALS.....1
        (MAXIMUM IS 10)
NS.....BLOCK SIZE FOR THE LARGE CAPACITY SOLVER.....96
NPBC...NUMBER OF NODAL POINTS WITH BOUND. COND.....8
NPBC2...NUMBER OF NODAL POINTS WITH PREDEF. DISPL.....0
NCLD...NUMBER OF NODAL POINTS WITH CONC. LOAD.....4
NN.....NUMBER OF BLOCKS PER COLUMN.....6
MM.....NUMBER OF BLOCKS PER ROW.....2
NPCL...NUMBER OF NODES PER ELEMENT.....32
NREC7...NUMBER OF RECORDS OF FILE NO 7.....12
```

\*\*\*\*\* JCL CARDS \*\*\*\*\*

```
//GO.FT07F001 DD UNIT=SYSDA,SPACE=(6144,(0144,1)),DISP=(NEW,DELETE)
//GO.FT08F001 DD UNIT=SYSDA,SPACE=(6144,(0096,1)),DISP=(NEW,DELETE)
//GO.FT09F001 DD UNIT=SYSDA,SPACE=(0128,(0008,1)),DISP=(NEW,DELETE)
//GO.FT10F001 DD UNIT=SYSDA,SPACE=(0024,(0172,1)),DISP=(NEW,DELETE)
//GO.FT12F001 DD UNIT=SYSDA,SPACE=(0056,(0172,1)),DISP=(NEW,DELETE)
//GO.FT13F001 DD UNIT=SYSDA,SPACE=(0056,(0172,1)),DISP=(NEW,DELETE)
//GO.FT20F001 DD UNIT=SYSDA,SPACE=(0768,(0006,1)),DISP=(NEW,DELETE)
//GO.FT21F001 DD UNIT=SYSDA,SPACE=(0024,(0192,1)),DISP=(NEW,DELETE)
//GO.FT14F001 DD UNIT=SYSDA,SPACE=(0032,(0004,1)),DISP=(NEW,DELETE)
//GO.FT15F001 DD UNIT=SYSDA,SPACE=(0016,(0008,1)),DISP=(NEW,DELETE)
//GO.FT17F001 DD UNIT=SYSDA,SPACE=(0032,(0008,1)),DISP=(NEW,DELETE)
//GO.FT19F001 DD UNIT=SYSDA,SPACE=(0480,(0008,1)),DISP=(NEW,DELETE)
```

\*\*\*\*\* DEFINE FILE STATEMENTS \*\*\*\*\*

```
DEFINE FILE 7( 144,1536,U,17),P( 96,1536,U,18),
1      10( 172, 6,U,110), DSK00100
2      12( 172, 14,U,112),13( 172, 14,U,113), DSK00200
3      20( 6, 142,U,120),21( 192, 8,U,121) DSK00210
                                           DSK00220
```

\*\*\*\*\* GENERAL FILE DATA \*\*\*\*\*

NRX = NUMBER OF RECORDS OF FILE X  
LFPX = LENGTH OF RECORD OF FILE X IN BYTES  
LPWX = --- --- --- WORDS

NR7 = 144	NR9 = 96	NR9 = 8	NR10 = 172	NR12 = 172
LRB7 = 6144	LRB9 = 6144	LRB9 = 128	LRB10 = 24	LRB12 = 56
LRW7 = 1536	LPW9 = 1536		LRW10 = 6	LRW12 = 14
NREC7 = 12				
NR13 = 172	NR14 = 4	NR15 = 8	NR16 = 0	NR17 = 8
LRB13 = 56	LRB14 = 32	LRB15 = 16	LRB16 = 0	LRB17 = 32
LRW13 = 14				
NR19 = 8	NR20 = 6	NR21 = 192	NR22 = 0	
LRB19 = 480	LRB20 = 768	LRB21 = 24	LRB22 = 0	
	LRW20 = 192	LRW21 = 6		

NOTE : IF DATA OF FILES NO 16 OR 22 ARE ZERO THESE FILES ARE NOT REQUIRED



# APPENDIX F

## DECK STRUCTURE FOR COMPUTER PROGRAMS IN IBM 360 OS

### A. TRISOP - QUADRATIC ELEMENTS

```

JOB CARD
// EXEC FORTHCLG,REGION.FORT=188K,REGION.LINK=118K,REGION.GO=220K
//SYSPRINT DD SYSOUT=A,SPACE=(CYL,(3,1))
//FORT.SYSIN DD *

```

\*\*\*\*\*

#### FORTRAN DECK

```

//GO.FT07F001 DD UNIT=SYSDA,SPACE=(LRB7,37,1),DISP=(NEW,DELETE)
//GO.FT08F001 DD UNIT=SYSDA,SPACE=(LRB8,38,1),DISP=(NEW,DELETE)
//GO.FT09F001 DD UNIT=SYSDA,SPACE=(00080,39,1),DISP=(NEW,DELETE)
//GO.FT10F001 DD UNIT=SYSDA,SPACE=(00024,40,1),DISP=(NEW,DELETE)
//GO.FT11F001 DD UNIT=SYSDA,SPACE=(00056,41,1),DISP=(NEW,DELETE)
//GO.FT12F001 DD UNIT=SYSDA,SPACE=(00056,42,1),DISP=(NEW,DELETE)
//GO.FT13F001 DD UNIT=SYSDA,SPACE=(00032,43,1),DISP=(NEW,DELETE)
//GO.FT14F001 DD UNIT=SYSDA,SPACE=(00016,44,1),DISP=(NEW,DELETE)
//GO.FT15F001 DD UNIT=SYSDA,SPACE=(00004,45,1),DISP=(NEW,DELETE)
//GO.FT16F001 DD UNIT=SYSDA,SPACE=(00032,46,1),DISP=(NEW,DELETE)
//GO.FT17F001 DD UNIT=SYSDA,SPACE=(00004,47,1),DISP=(NEW,DELETE)
//GO.FT19F001 DD UNIT=SYSDA,SPACE=(00480,49,1),DISP=(NEW,DELETE)
//GO.FT20F001 DD UNIT=SYSDA,SPACE=(00024,50,1),DISP=(NEW,DELETE)
//GO.FT21F001 DD UNIT=SYSDA,SPACE=(00024,51,1),DISP=(NEW,DELETE)
//GO.FT22F001 DD UNIT=SYSDA,SPACE=(00024,52,1),DISP=(NEW,DELETE)

```

\*\*\*\*\*

#### DATA CARDS

\*\*\*\*\*





B. TRISOP - CUBIC ELEMENTS

```

JOB CARD
// EXEC FORTHCLG,REGION.FORT=188K,REGION.LINK=118K,REGION.GO=347K
//SYSPRINT DD SYSOUT=A,SPACE=(CYL,(3,1))
//FORT.SYSIN DD *
```

```

*****
*****
*****
*****
*****
*****
```

FORTTRAN DECK

```

//GO.FT07F001 DD UNIT=SYSDA,SPACE=(LRB7, (VR7,1)),DISP=(NEW,DELETE)
//GO.FT08F001 DD UNIT=SYSDA,SPACE=(LRB8, (VR8,1)),DISP=(NEW,DELETE)
//GO.FT09F001 DD UNIT=SYSDA,SPACE=(00128, (VR9,1)),DISP=(NEW,DELETE)
//GO.FT10F001 DD UNIT=SYSDA,SPACE=(00024, (VR10,1)),DISP=(NEW,DELETE)
//GO.FT11F001 DD UNIT=SYSDA,SPACE=(00056, (VR11,1)),DISP=(NEW,DELETE)
//GO.FT12F001 DD UNIT=SYSDA,SPACE=(00032, (VR12,1)),DISP=(NEW,DELETE)
//GO.FT13F001 DD UNIT=SYSDA,SPACE=(00016, (VR13,1)),DISP=(NEW,DELETE)
//GO.FT14F001 DD UNIT=SYSDA,SPACE=(00032, (VR14,1)),DISP=(NEW,DELETE)
//GO.FT15F001 DD UNIT=SYSDA,SPACE=(00048, (VR15,1)),DISP=(NEW,DELETE)
//GO.FT16F001 DD UNIT=SYSDA,SPACE=(00072, (VR16,1)),DISP=(NEW,DELETE)
//GO.FT17F001 DD UNIT=SYSDA,SPACE=(00096, (VR17,1)),DISP=(NEW,DELETE)
//GO.FT18F001 DD UNIT=SYSDA,SPACE=(00120, (VR18,1)),DISP=(NEW,DELETE)
//GO.FT19F001 DD UNIT=SYSDA,SPACE=(00144, (VR19,1)),DISP=(NEW,DELETE)
//GO.FT20F001 DD UNIT=SYSDA,SPACE=(00168, (VR20,1)),DISP=(NEW,DELETE)
//GO.FT21F001 DD UNIT=SYSDA,SPACE=(00192, (VR21,1)),DISP=(NEW,DELETE)
//GO.FT22F001 DD UNIT=SYSDA,SPACE=(00216, (VR22,1)),DISP=(NEW,DELETE)
//GO.SYSIN DD *
```

```

*****
*****
*****
*****
*****
*****
```

DATA CARDS



### C. AUXILIARY PROGRAM JCL

```
// JOB CARD
// EXEC FRTCALCP
//FCRT.SYSIN DD *

      ***
      MAIN PROGRAM - JCL
      (FORTRAN)
      ***

/*
//ASM.SYSIN DD *

      ***
      SUBPROGRAM EXDICC
      (ASSEMBLY LANGUAGE)
      ***

//GO.FT08F001 DD UNIT=SYSDA,SPACE=(4,(5,1)),DISP=(NEW,DELETE)
//GC.SYSIN DD *

      ***
      DATA CARDS
      ***
```



## APPENDIX G

### COMPUTER LISTINGS FOR TRISOP

In this Appendix two computer listings for TRISOP are presented, one for quadratic elements and one for cubic elements.

The two listings are basically the same. The minor existing differences are:

#### 1. Core Space Requirements

The core space requirements for cubic elements are higher than those for quadratic. Those requirements are reflected in the dimensioning of arrays appearing in COMMON and DIMENSION statements.

#### 2. Input of Connectivity Data

The requirement for two headings and also reading of connectivity matrix in two parts for cubic elements dictates an alteration of the corresponding part of subroutine INPUT.

#### 3. Subroutine DISK

Entries RDISK1 and WDISK1 differ in quadratic and cubic elements because optimization of transfer time for element stiffness matrices requires a different partitioning of these matrices into data file records for each case. Because of the similitude of the two programs the listing of TRISOP for cubic elements is not presented complete. Only the parts that differ from the listing for quadratic elements are shown.



Listing for subroutine CUB is not given in this Appendix since there is a choice of different subroutines each corresponding to integration by use of a different number of integration points. Computer listings for these subroutines are presented in Appendix I.





MAI00010  
MAI00015  
MAI00020  
MAI00025  
MAI00030  
MAI00035  
MAI00040  
MAI00050  
MAI00060  
MAI00070  
MAI00080  
MAI00090  
MAI00100  
MAI00110  
MAI00120  
MAI00130  
MAI00140  
MAI00150  
MAI00160  
MAI00170  
MAI00180  
MAI00190  
MAI00200  
MAI00210

```
COMMON /NB1/ NEL, NDPT, NPCL, NDF, NEQ, NN, MM, NS, NCOUNT, NST, NSTF, NCCLD,
1 NPBC, NPBC2, NBCL, NBCH, NMA7, NREC7, KEL
COMMON /NB3/ NR7, LRB7, LRW7, NNRC, NR8, LRB8, LRWB8, LRWB9, NR9, NR10, LRB10,
1 LRW10, NR12, LRB12, LRW12, NR13, LRB13, LRWB13, LRWB14, NR14, NR15, LRB15, NR
2, LRB16, NR17, LRB17, NR19, LRB19, NR20, LRB20, LRWB20, LRWB21, LRWB21,
3 NR22, LRB22
DIMENSION NFR(36)
EQUIVALENCE (NFR(1), NR7)
```

NOTES, LISTEN NER (36)

DIFFERENTIAL EQUATIONS (NR7), NR7)

READ(5,1000) NFR  
NREC7=NNRC

## INITIALIZATION OF THE PROGRAM

CALL MAINE

```
FORMAT(7I10)
STOP
END
```

[illegible]

SUBROUTINE MAINE

```

IMPLICIT REAL*8 (A-H,O-Z)
COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
INPRC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDEP,PULINE,NREC7,KEL
1COMMON /B2/ ELAST(6,6),COORD(3),COREL(20,3),UJNT(3),SSSS1(7),
1SSNN1(7)
1DIMENSIONION SSSS(6),SSNN(6)
EQUIVALENCE (SSSS(1),SSSS1(1)),(SSNN(1),SSNN1(1))

```

```
CCCCC INPUT
10 CALL SETTIME(0)*0.01
```

## INPUT

```
10 CALL SETIME(0)*0.01
   CLOCK=ITIME
   READ(5,950) CHKWD
```







CC

FORMATION OF LOAD VECTOR

```
CALL SETTIME
CLOCK=ITIME(0)*0.01
CALL FLOAD
CALL GETTIME(IET)
CPUTM=IET*0.000026
WRITE(6,6000)
CLOCK=ITIME(0)*0.01-CLOCK
WRITE(6,8000) CLOCK,CPUTM
```

CC

APPLICATION OF BOUNDARY CONDITIONS

SELECT THE APPROPRIATE SEGMENT (1) OR (2) DEPENDING ON  
WHETHER THE B.C.'S INCLUDE PREDEFINED DISPLACEMENTS OR  
NOT

IF(NPBC2.NE.0) GO TO 100

(1) THIS SEGMENT REFERS TO B.C.'S WITH NO PREDEFINED  
DISPLACEMENTS

```
CALL SETTIME
CLOCK=ITIME(0)*0.01
CALL BCOND
CALL GETTIME(IET)
CPUTM=IET*0.000026
WRITE(6,7000)
CLOCK=ITIME(0)*0.01-CLOCK
WRITE(6,8000) CLOCK,CPUTM
GO TO 200
```

(2) THIS SEGMENT REFERS TO B.C.'S INCLUDING PREDEFINED  
DISPLACEMENTS

```
100 CALL SETTIME
CLOCK=ITIME(0)*0.01
CALL BCOND2
CALL GETTIME(IET)
CPUTM=IET*0.000026
WRITE(6,7500)
CLOCK=ITIME(0)*0.01-CLOCK
WRITE(6,8000) CLOCK,CPUTM
```

C

SMA00670  
SMA00680  
SMA00690  
SMA00700  
SMA00710  
SMA00720  
SMA00730  
SMA00740  
SMA00750  
SMA00760  
SMA00770  
SMA00780  
SMA00790  
SMA00800  
SMA00810  
SMA00820  
SMA00830  
SMA00840  
SMA00850  
SMA00860  
SMA00870  
SMA00880  
SMA00890  
SMA00900  
SMA00910  
SMA00920  
SMA00930  
SMA00940  
SMA00950  
SMA00960  
SMA00970  
SMA00980  
SMA00990  
SMA01000  
SMA01010  
SMA01020  
SMA01030  
SMA01040  
SMA01050  
SMA01060  
SMA01070  
SMA01080  
SMA01090  
SMA01100  
SMA01110  
SMA01120  
SMA01130  
SMA01140









SMA01630  
SMA01640  
SMA01650  
SMA01660  
SMA01670  
SMA01680  
SMA01690  
SMA01700  
SMA01710  
SMA01720  
SMA01730  
SMA01740  
SMA01750  
SMA01760  
SMA01770  
SMA01780  
SMA01790  
SMA01800  
SMA01810  
SMA01820  
SMA01830  
SMA01840  
SMA01850  
SMA01860  
SMA01870  
SMA01880  
SMA01890  
SMA01900  
SMA01910  
SMA01920  
SMA01930  
SMA01940  
SMA01950  
SMA01960  
SMA01970  
SMA01980  
SMA01990  
SMA02000  
SMA02010  
SMA02020  
SMA02030  
SMA02040  
SMA02050  
SMA02060  
SMA02070  
SMA02080  
SMA02090  
SMA02100

MATERIALS PRESENT IN THE STRUCTURE

```

NMAT1=0
REWIND 16
DO 500 I=1,NEL
  READ(19) COREL
  READ(16) NCON21
  IF(NMAT1.EQ.NCON21) GO TO 450
  NMAT1=NCON21
  CALL ELPROP(NMAT1)
  CALL STRESS(I)
450 CONTINUE
500 GO TO 650

```

(2) THIS SEGMENT IS USED FOR THE CASE OF ONE MATERIAL

```

550 DO 600 I=1,NEL
  READ(19) COREL
  CALL STRESS(I)
600

```

\*\*\*\*\*

COMPUTATION OF AVERAGE NODAL STRESSES AND STRAINS

(1) AVERAGE STRESSES

```

650 NPAGE=NDPT/NLINE
  NL=NPAGE*NLINE
  NL1=NL+1
  NLAST=NDPT-NL
  IF(NPAGE.EQ.0) GO TO 710
  DO 705 I=1,NPAGE
    WRITE(6,9700)
    I1=(I-1)*NLINE+1
    I2=I1+NLINE-1
    DO 705 J=I1,I2
      CALL RDISKS(J)
    DO 700 K=1,9
      SSSS1(K)=SSSS1(K)/SSSS1(7)
700 SSSS1(K)=SSSS1(K)
705 WRITE(6,9600) J,SSSS
    IF(NLAST.EQ.0) GO TO 730
710 WRITE(6,9700)
    DO 725 I=NL1,NDPT
      CALL RDISKS(I)
    DO 720 K=1,6
      SSSS1(K)=SSSS1(K)/SSSS1(7)
720 SSSS1(K)=SSSS1(K)
725 WRITE(6,9600) I,SSSS

```



```

730 CONTINUE
C
C      (2) AVERAGE STRAINS
C
      IF(NPAGE.EQ.0) GO TO 745
      DO 740 I=1,NPAGE
      WRITE(6,9500)
      I1=(I-1)*NLINE+1
      I2=I1+NLINE-1
      DO 740 J=I1,I2
      CALL RDISKNI(J)
      DO 735 K=1,6
      SSNN1(K)=SSNN1(K)/SSNN1(7)
735  SSNN1(K)=SSNN1(K)/SSNN1(7)
740  WRITE(6,9600) J,SSNN
      IF(NLAST.EQ.0) GO TO 760
745  WRITE(6,9500)
      DO 755 I=NL1,NDPT
      CALL RDISKNI(I)
      DO 750 K=1,6
      SSNN1(K)=SSNN1(K)/SSNN1(7)
750  SSNN1(K)=SSNN1(K)/SSNN1(7)
755  WRITE(6,9600) I,SSNN
760  CONTINUE
      CALL GETIME(IET)
      CPUTIME=IET*0.000026
      WRITE(6,9200)
      CLOCK=ITIMF(0)*0.01-CLOCK
      WRITE(6,8000) CLOCK,CPUTM
C
C*****
C      GO TO 10
C
9500  FORMAT(1A8)
1000  FORMAT(5X,'*****')
2000  FORMAT(5X,'*****')
3000  FORMAT(5X,'*****')
4000  FORMAT(5X,'*****')
5000  FORMAT(5X,'*****')
6000  FORMAT(5X,'*****')
7000  FORMAT(5X,'*****')
7500  FORMAT(5X,'*****')
9000  FORMAT(5X,'*****')
9100  FORMAT(5X,'*****')
9200  FORMAT(5X,'*****')
8000  FORMAT(5X,7H TIME =,1F7.2,8H SECONDS,6H(CPU =,1F7.2,9H SECONDS)//)
9500  FORMAT(1H1,/, ' A-V-E-R-A-G-E S-T-R-A-I-N-S AT THE JOINTS ',
1//,2X,'SNX',12X,'SNY',12X,'SNZ',12X,'SNXY',11X,'SNYZ',11X,'SNZX',//)
29X,'SNX',12X,'SNY',12X,'SNZ',11X,'SNZX',//)
SMA02110
SMA02120
SMA02130
SMA02140
SMA02150
SMA02160
SMA02170
SMA02180
SMA02190
SMA02200
SMA02210
SMA02220
SMA02230
SMA02240
SMA02250
SMA02260
SMA02270
SMA02280
SMA02290
SMA02300
SMA02310
SMA02320
SMA02330
SMA02340
SMA02350
SMA02360
SMA02370
SMA02380
SMA02390
SMA02400
SMA02410
SMA02420
SMA02430
SMA02440
SMA02450
SMA02460
SMA02470
SMA02480
SMA02490
SMA02500
SMA02510
SMA02520
SMA02530
SMA02540
SMA02550
SMA02560
SMA02570
SMA02580
SMA02590

```







```

C** NSTF=NST*NST
C** NSB=6*NST
C** NN=(NEQ+NS-1)/NS
C**
C** *****
C** STOP TRAPS FOR WRONG CHARACTERISTICS OF FILES (EXCEPT NR7)
C** STOP TRAP FOR WRONG NUMBER OF MATERIALS
C**
C** CALL ERROR1(NMAT)
C** *****
C**
C** CONNECTIVITY MATRIX
C**
C** READ(5,1000) TITLE
C** READ(5,1000) FMT
C** WRITE(6,1100)
C** WRITE(6,1000) TITLE
C** NBAND=0
C**
C** SELECT THE APPROPRIATE SEGMENT (1) OR (2) DEPENDING ON
C** WHETHER ONE OR MORE MATERIALS ARE PRESENT IN THE STRUCTURE
C**
C** IF(NMAT.EQ.1) GO TO 150
C**
C** (1) THIS SEGMENT IS USED FOR THE CASE OF MORE THEN ONE
C** MATERIALS PRESENT IN THE STRUCTURE
C**
C** DO 100 I=1,NEL
C** READ(5,FMT) NCON,NCON21
C** WRITE(6,FMT) NCON,NCON21
C** WRITE(9) NCON
C** WRITE(16) NCON21
C**
C** COMPUTATION OF HALF BANDWIDTH
C**
C** NPELM=NPEL-1
C** DO 100 J=1,NPELM
C** JP=J+1
C** DO 100 K=JP,NPEL
C** NBAND=MAXO(NBAND,IABS(NCON(J)-NCON(K)))
C** GO TO 200
C**
C** 100
C**
C** (2) THIS SEGMENT IS USED FOR THE CASE OF ONE MATERIAL
C**
C** 150 DO 160 I=1,NEL
C**

```













```

READ(5,FMT) NBC
WRITE(6,FMT) NBC
NBC=NRC(1)
NBC(1)=NDF*NRC(1)-NDF
WRITE(15) NBC
NPBC=NPRC-1
DO 600 I=2,NPRCL
  READ(5,FMT) NBC
  WRITE(6,FMT) NBC
  NBC(1)=NDF*NRC(1)-NDF
  WRITE(15) NBC
  READ(5,FMT) NBC
  WRITE(6,FMT) NBC
  NBCH=NRC(1)
  NBC(1)=NDF*NBC(1)-NDF
  WRITE(15) NBC
600

      IF THE R.C.'S DO NOT INCLUDE PREDEFINED DISPLACEMENTS
      SKIP NEXT SEGMENT

      IF(NPBC2.EQ.0) RETURN

      (2) PREDEFINED DISPLACEMENTS

      READ(5,1000) TITLE
      WRITE(6,1000) TITLE
      WRITE(6,1100)
      READ(5,1000) FMT
      DO 650 I=1,NPBC2
        READ(5,FMT) NDBC,BCON
        WRITE(6,FMT) NDBC,BCON
        WRITE(22) BCON
650

      RETURN

      FORMAT(10A8)
      FORMAT(///)
      1000 FORMAT(10I5)
      2000 FORMAT(//,
      3000 15X,NELT... TOTAL NUMBER OF ELEMENTS... I3/,
      25X,NDPT... TOTAL NUMBER OF NODAL POINTS... I3/,
      35X,NMAT... NUMBER OF DIFFERENT MATERIALS... I3/,
      45X,NMAX... (MAXIMUM SIZE FOR THE LARGE CAPACITY SOLVER... I3/,
      55X,NNS... BLOCK SIZE FOR THE LARGE CAPACITY SOLVER... I3/,
      65X,NPRC... NUMBER OF NODAL POINTS WITH BOUND. COND... I3/,
      75X,NPBC2... NUMBER OF NODAL POINTS WITH PREDEF. DISPL... I3/,

```

```

INP01820
INP01830
INP01840
INP01850
INP01860
INP01870
INP01880
INP01890
INP01900
INP01910
INP01920
INP01930
INP01940
INP01950
INP01960
INP01970
INP01980
INP01990
INP02000
INP02010
INP02020
INP02030
INP02040
INP02050
INP02060
INP02070
INP02080
INP02090
INP02100
INP02110
INP02120
INP02130
INP02140
INP02150
INP02160
INP02170
INP02180
INP02190
INP02200
INP02210
INP02220
INP02230
INP02240
INP02250
INP02260
INP02270
INP02280
INP02290

```

```

C
C
C
C
C
C
C
C

```









```

SUBROUTINE FSTF
  IMPLICIT REAL*8(A-H,O-Z)
  COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
  1NPBC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDFP,NLINE,NRECT,KEL
  COMMON /B2/ ELAST(6,6),COORD(3),COREL(3,3),UJNT(3),SSSS1(7),
  1SSNN1(7)
  COMMON /B3/ AK1(60,60),AK2(30,60),AK3(30,60),RB1(60),RB2(60),
  1RB3(60)
  DIMENSION STK(3600),AK(3600),B(360)
  EQUIVALENCE (AK1(1,1),STK(1)),(AK2(1,1),AK(1)),(AK3(1,1),B(1))
  SELECT THE APPROPRIATE SEGMENT (1) OR (2) DEPENDING ON
  WHETHER ONE OR MORE MATERIALS ARE PRESENT IN THE STRUCTURE
  REWIND 19
  IF(NMAT.EQ.1) GO TO 300
  (1) THIS SEGMENT IS USED FOR THE CASE OF MORE THEN ONE
  MATERIALS PRESENT IN THE STRUCTURE
  REWIND 16
  N2=-1
  DO 200 I=1,NEL
  READ(19) COREL
  READ(16) NCON21
  N=NCON21
  IF(N.EQ.N2) GO TO 100
  CALL ELPROP(N)
  N2=N
  100 CALL CUR
  CALL WDISK1(I)
  200 CONTINUE
  RETURN
  (2) THIS SEGMENT IS USED FOR THE CASE OF ONE MATERIAL
  CALL ELPROP(1)
  DO 400 I=1,NEL
  READ(19) COREL
  CALL CUR
  CALL WDISK1(I)
  400 CONTINUE
  RETURN
  END

```

```

FST00020
FST00030
FST00040
FST00050
FST00060
FST00070
FST00080
FST00090
FST00100
FST00110
FST00120
FST00130
FST00140
FST00150
FST00160
FST00170
FST00180
FST00190
FST00200
FST00210
FST00220
FST00230
FST00240
FST00250
FST00260
FST00270
FST00280
FST00290
FST00300
FST00310
FST00320
FST00330
FST00340
FST00350
FST00360
FST00370
FST00380
FST00390
FST00400
FST00410
FST00420
FST00430
FST00440
FST00450
FST00460

```



SUBROUTINE CUB

LISTINGS OF SUBROUTINES PERFORMING NUMERICAL INTEGRATION BY USE OF TWO, THREE, FOUR, OR FIVE GAUSS POINTS ARE SHOWN IN APPENDIX

SUBROUTINE FORMK(X,Y,Z,INDIC)

IMPLICIT REAL\*8(A-H,O-Z)

```

11PCLC1/ NBCL/ NDEL, NDF, NEQ, NN, M3, NS, NCOUNT, NST, NSTF, NCLD,
COMMON /NB1/ NEL, NDPT, NDEL, NDF, NEQ, NN, M3, NS, NCOUNT, NST, NSTF, NCLD,
11NPBC, NPBC2, NBCL, NBCH, NMAT, NCUN21, NDOP, KLINE, NREC7, KEL
COMMON /R2/ ELAST(6,6), COORD(3), COREL(20,3), UJNT(3), SSSS1(7),
11SSNN1(7)
COMMON /B3/ AK1(60,60), AK2(60,60), AK3(60,60), RB1(60), RB2(60),
11RB3(60)
DIMENSION STK(60,60), AK(60,60), B(6,60), B1(60,6), W1(3,20),
11DNX(3,20), AJIN(3,3), AJ(3,3), CORDG(20,3)
EQUIVALENCE (STK(1,1), AK1(1,1)), (AK(1,1), AK2(1,1)), (B(1,1),
11AK3(1,1)), (B1(1,1), AK3(1,7)), (W1(1,1), RB3(1))
DATA CORDG/1.000,0.000,-1.000,-1.000,0.000,1.000,1.000,1.000,
111.000,-1.000,-1.000,1.000,1.000,0.000,-1.000,-1.000,0.000,
211.000,1.000,1.000,1.000,1.000,0.000,-1.000,-1.000,0.000,
311.000,1.000,-1.000,-1.000,1.000,1.000,1.000,-1.000,-1.000,
411.000,0.000,1.000,1.000,1.000,1.000,1.000,1.000,1.000,-1.000,
501.000,0.000,0.000,-1.000,-1.000,-1.000,-1.000,-1.000,-1.000/

```

# GENERAL FORMULAE FOR COMPUTATION OF DERIVATIVES OF SHAPE FUNCTIONS WITH RESPECT TO 'XI', 'ETA', 'ZETA'



```

C C C      (1) CORNER NODES
C          DF(C,D,E,X1,Y1,Z1)=X1*(1.0D0+D*Y1)*(1.0D0+E*Z1)*(2.0D0*C*X1+D*Y1+
            1E*Z1-1.0D0)/8.0D0
C C C      (2) MIDSIDE NODES
C          D2(C,D,E,    Y1,Z1)=-C*(1.0D0+D*Y1)*(1.0D0+E*Z1)/2.0D0
C          D4(C,   E,    Y1,Z1)=(1.0D0-C*C)*Y1*(1.0D0+E*Z1)/4.0D0
C *****
C ***** COMPUTATION OF DERIVATIVES OF SHAPE FUNCTIONS WITH RESPECT
C ***** TO 'XI','ETA','ZETA' EVALUATED AT THE POINTS OF INTEGRATION *****
C *****
C          (1) CORNER NODES :      XI= + 0; - 1
C                                ZETA= + 0; - 1
C                                ETA= + 0; - 1
C
C                                NODE # : 1,3,5,7,13,15,17,19
C
C          DO 100 I=1,2
C             II=12*(I-1)+1
C             IT=II+6
C             DO 100 J=1,IT,2
C                X1=CORDG(J,1)
C                Y1=CORDG(J,2)
C                Z1=CORDG(J,3)
C                W1(1,J)=DF(X,Y,Z,X1,Y1,Z1)
C                W1(2,J)=DF(Y,Z,X,Y1,Z1,X1)
C                W1(3,J)=DF(Z,X,Y,Z1,X1,Y1)
C          100
C *****
C ***** (2) MIDSIDE NODES :      XI= 0      ZETA= + 0; - 1
C                                     ETA= + 0; - 1
C
C                                     NODE # : 2,6,14,18
C
C          DO 200 K=1,2
C             II={K-1}*12+2
C             IT=II+4
C             DO 200 L=1,IT,4
C                X1=CORDG(L,1)
C                Y1=CORDG(L,2)
C                Z1=CORDG(L,3)
C                W1(1,L)=D2(X,Y,Z,    Y1,Z1)
C                W1(2,L)=D4(X,Y,Z,    Y1,Z1)

```













FMK01710  
FMK01720  
FMK01730  
FMK01740  
FMK01750  
FMK01760  
FMK01770  
FMK01780  
FMK01790  
FMK01800  
FMK01810  
FMK01820  
FMK01830  
FMK01840  
FMK01850  
FMK01860  
FMK01870  
FMK01880  
FMK01890  
FMK01900  
FMK01910  
FMK01920

MULTIPLICATION OF MATRICES AND DETERMINANT OF JACOBIAN IN  
THE INTEGRANT OF STIFFNESS MATRIX FORMULA

```

DO 800 I=1,NST
DO 750 J=1,3
DO 750 K=1,3
DO 750 L=4,6
750 B1(I,J)=B1(I,J)+B(K,I)*ELAST(K,J)
800 B1(I,L)=B1(I,L)+B(L,I)*ELAST(L,L)
DO 900 I=1,NST
DO 900 J=1,NST
DO 900 K=1,6
AK(I,J)=0.0D0
900 AK(I,J)=AK(I,J)+B1(I,K)*B(K,J)
DO 1000 I=1,NST
DO 1000 J=1,NST
AK(I,J)=AK(I,J)*DTJ
1000 AK(J,I)=AK(I,J)

```

RETURN  
END

\*\*\*\*\*

SUBROUTINE MERGE

MRG000020  
MRG000030  
MRG000040  
MRG000050  
MRG000060  
MRG000070  
MRG000080  
MRG000090  
MRG000100  
MRG000110  
MRG000120  
MRG000130  
MRG000140  
MRG000150  
MRG000160  
MRG000170  
MRG000180  
MRG000190  
MRG000200

```

IMPLICIT REAL*8(A-H,O-Z)
COMMON /NB1/NEL,NDPT,NPEL,NOF,NEQ,NN,NM,NS,NCOUNT,NST,NSTF,NCLD,
1INPBC,NPBC2,NBCL,NBCH,NMAC,NBC(4)
COMMON /NB2/ NCON(20),NBC(4)
COMMON /B3/ AK1(60,60),AK2(60,60),AK3(60,60),RB1(60),RB2(60),
1RB3(60)
DIMENSION A(60,60),S(60,60),B(3600),T(3600),LM(20)
EQUIVALENCE (AK2(1,1),A(1,1),B(1)),(AK1(1,1),S(1,1),T(1)),
1(LM(1),RB1(1))

```

NODE NUMBERS REFERRED TO IN THIS SUBROUTINE BELONG TO THE  
NUMBERING SYSTEM OF THE TOTAL STRUCTURE AND NOT LOCAL  
ELEMENT NUMBERING

NTRK(I,J)=SEQUENCE NUMBER OF BLOCK POSITIONED IN ROW 'I'  
AND COLUMN 'J'

CCCCCCCC

C

CCCC

C

CCCC



```

C      BLOCKS ARE COUNTED ROW WISE
C
C      NTRK(I,J)=(I-1)*(MM)+J
C      ZRQ=0.000
C
C      FILL ALL BLOCKS OF TOTAL STIFFNESS MATRIX IN ROW SEQUENCE
C      BY PICKING THE PROPER ELEMENTS FROM THE INDIVIDUAL
C      ELEMENT STIFFNESS MATRICES
C
C      DO 400 I=1,NN
C      DO 400 J=1,MM
C      DO 100 I1=1,NS
C      DO 100 J1=1,NS
C      100 A(I1,J1)=ZRQ
C
C      EXAMINE ALL ELEMENT STIFFNESS MATRICES FOR TERMS BELONGING
C      TO THE BLOCK (I,J) AND TRANSFER THEM ACCORDINGLY
C
C      REWIND 9
C      DO 300 K=1,NEL
C      READ(9) NCON
C
C      DETERMINE THE LOWEST AND HIGHEST NODE NUMBER FOR EACH
C      ELEMENT
C
C      DO 200 L=1,NPEL
C      LM(L)=NDF*NCON(L)-NDF
C      LMIN=5000
C      LMAX=0
C      DO 210 I2=1,NPEL
C      LMIN=MINO(LMIN,LM(I2))
C      LMAX=MAXO(LMAX,LM(I2))
C      LMIN=LMIN+1
C      LMAX=LMAX+1
C      NSI=NS*I
C      NSIM=NSI-NS
C
C      IF NO NODE OF THE ELEMENT EXAMINED BELONGS TO BLOCK (I,J)
C      GO TO THE NEXT ELEMENT
C
C      IF(LMIN.GT.NSI) GO TO 300
C      IF(LMAX.LE.NSIM) GO TO 300
C
C      IF ONE OR MORE NODES OF THE ELEMENT EXAMINED BELONG TO
C      BLOCK (I,J) READ THE ELEMENT STIFFNESS MATRIX FROM DISK
C
C      CALL RDISK1(K)
C
C      MRG000210
C      MRG000220
C      MRG000230
C      MRG000240
C      MRG000250
C      MRG000260
C      MRG000270
C      MRG000280
C      MRG000290
C      MRG000300
C      MRG000310
C      MRG000320
C      MRG000330
C      MRG000340
C      MRG000350
C      MRG000360
C      MRG000370
C      MRG000380
C      MRG000390
C      MRG000400
C      MRG000410
C      MRG000420
C      MRG000430
C      MRG000440
C      MRG000450
C      MRG000460
C      MRG000470
C      MRG000480
C      MRG000490
C      MRG000500
C      MRG000510
C      MRG000520
C      MRG000530
C      MRG000540
C      MRG000550
C      MRG000560
C      MRG000570
C      MRG000580
C      MRG000590
C      MRG000600
C      MRG000610
C      MRG000620
C      MRG000630
C      MRG000640
C      MRG000650
C      MRG000660
C      MRG000670
C      MRG000680

```









```

C
SUBROUTINE FLOAD
  IMPLICIT REAL*8(A-H,O-Z)
  COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MH,NS,NCOUNT,NST,NSTF,NCLD,
  1NPBC,NPBC2,NBCL,NMAT,NCOUN21,NDFP,NLINE,NREC7,KEL
  COMMON /B1/ ELDAT(10,3),CLOAD(4),BCON(1)
  COMMON /B3/ AK1(60,60),AK2(60,60),AK3(60,60),RB1(60),RB2(60),
  1RB3(60)
  DIMENSION A(60,60),S(60,60),R1(60),R2(60)
  EQUIVALENCE(A(1,1),AK1(1,1)),(S(1,1),AK2(1,1)),(R1(1),RB1(1)),
  1(R2(1),RB2(1))
  ZRO=0.000
  REWIND 14
  READ(14) CLOAD
  INDEX=1
  INCH=CLOAD(NDFP)*NDF
  EXAMINE EACH RIGHT HAND BLOCK AND POSITION IN IT ALL
  CORRESPONDING NODAL LOAD VECTOR:
  DO 1100 I=1,NN
  DO 100 J=1,NS
  100 R1(J)=ZRO
  IF(I*NS
  50 IF(INCH.GT.IH) GO TO 1000
  IR1=INCH-(I-1)*NS-NDF
  DO 150 K=1,NDF
  150 IR=IR1+K
  R1(IR)=CLOAD(K)
  IF THE LOAD POSITIONED IN THE BLOCK IS THE LAST ONE STOP
  THE PROCESS
  IF(INDEX.EQ.NCLD) GO TO 1200
  READ(14) CLOAD
  INDEX=INDEX+1
  INCH=CLOAD(NDFP)*NDF
  GO TO 50
  STORE COMPLETED BLOCK ON DISK
  1000 CALL WDSKR1(I)
  1100 CONTINUE
  RETURN
  1200 CALL WDSKR1(I)
  11=I+1

```

```

FLD000020
FLD000030
FLD000040
FLD000050
FLD000060
FLD000070
FLD000080
FLD000090
FLD000100
FLD000110
FLD000120
FLD000130
FLD000140
FLD000150
FLD000160
FLD000170
FLD000180
FLD000190
FLD000200
FLD000210
FLD000220
FLD000230
FLD000240
FLD000250
FLD000260
FLD000270
FLD000280
FLD000290
FLD000300
FLD000310
FLD000320
FLD000330
FLD000340
FLD000350
FLD000360
FLD000370
FLD000380
FLD000390
FLD000400
FLD000410
FLD000420
FLD000430
FLD000440
FLD000450
FLD000460
FLD000470
FLD000480
FLD000490

```



```

C
1350      IF (I1.GT.NN) RETURN
      DO 1300 K=I1,NN
      DO 1350 L=I,NS
      R1(L)=ZRO
      CALL WDSKR1(K)
      CONTINUE
      RETURN
      END

```

SUBROUTINE BCOND  
EC000020

```

DIMENSION AKW1(2000), AKW2(2000)
EQUIVALENCE (AKW1(1), AK1(1,1)), (AKW2(1), AK2(1,1))

```

$$\text{NTRK}(I, J) = (I-1) * (MM) + J$$

UU







```

C000820
C000830
C000840
C000850
C000860
C000870
C000880
C000890
C000900
C000910
C000920
C000930
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C000950
C000960
C000970
C000980
C000990
C001000
C001010
C001020
C001030
C001040
C001050
C001060
C001070
C001080
C001090
C001100
C001110
C001120
C001130
C001140
C001150
C001160
C001170
C001180
C001190
C001200
C001210
C001220
C001230
C001240
C001250
C001260
C001270
C001280
C001290

INDEX=INDEX+1
      IF THE NEW B.C. DOES NOT CORRESPOND TO DIAGONAL BLOCK (I,I)
      EXAMINE THE NEXT DIAGONAL BLOCK
      IF(NBC1.LE.IH) GO TO 110
      GO TO 140
      CALL WRDISK(NTK,AKW1)
      RETURN
130  CALL WRDISK(NTK,AKW1)
140  CONTINUE
150  RETURN
*****
      ENTRY 'BCOND2' IS USED ONLY FOR THE SOLUTION OF PROBLEMS
      WHOSE B.C.'S INCORPORATE PREDEFINED DISPLACEMENTS
*****
      ENTRY PCOND2
      DIMENSION BCON1(60,2)
      EQUIVALENCE (BCON1(1,1),AK3(1,1))
      ABIGN=1.0D20
      I1=1
      I5=0
      MMF=NN-MM+1
      DETERMINATION OF COLUMN 'IFR' AT WHICH THE FIRST B.C.
      APPLIES
      IFR=(NBCL*NS-1)/NS
      DETERMINATION OF ROW 'IR1' AT WHICH REDUCTION MUST START
      IR1=1
      IF(IFR.LE.MM) GO TO 160
      IR1=IFR-MM+1
      REDUCTION OF ALL ROWS AND CORRESPONDING RIGHT HAND VECTORS
      IN SEQUENCE STARTING WITH ROW 'IR1'
*****
C000820
C000830
C000840
C000850
C000860
C000870
C000880
C000890
C000900
C000910
C000920
C000930
C000940
C000950
C000960
C000970
C000980
C000990
C001000
C001010
C001020
C001030
C001040
C001050
C001060
C001070
C001080
C001090
C001100
C001110
C001120
C001130
C001140
C001150
C001160
C001170
C001180
C001190
C001200
C001210
C001220
C001230
C001240
C001250
C001260
C001270
C001280
C001290

```





```

C*** DO 900 I=IR1,NN
C   IFR1=IFR-I+1
C   IF(IFR1.GT.MM) GO TO 900
C   REWIND 15
C   REWIND 22
C
C   READ THE RIGHT HAND VECTOR CORRESPONDING TO ROW 'I'
C
C   NTKR1=I
C   CALL RDSKR1(NTKR1)
C
C   READ THE FIRST NOT APPLIED B.C.
C
C   DO 200 L=1,I1
C     READ(15) NBC
C     NBC1=NBC(1)+NDF
C     NBC2=NBC1/NDF
C
C     SET THE POINTER ON FILE 22 AT THE FIRST NOT YET APPLIED
C     PREDEFINED DISPLACEMENT
C
C     IF(15.EQ.0) GO TO 203
C     DO 202 L=1,I5
C       READ(22) BCON
C
C       DETERMINATION OF FIRST BLOCK OF ROW 'I' TO BE REDUCED AND
C       TRANSFER OF CONTROL TO THE APPROPRIATE SEGMENT
C
C       SELECT THE PROPER SEGMENT DEPENDING ON WHETHER THE FIRST
C       BLOCK TO BE REDUCED IS DIAGONAL OR NOT
C
C       IFR1=IFR
C       J3=2
C       IF(IFR.NE.I) GO TO 500
C
C       REDUCTION OF DIAGONAL BLOCK AND RIGHT HAND BLOCK OF ROW 'I'
C
C       I4=0
C       IH=I*NS

```



```

C      READ THE 'I' DIAGONAL BLOCK
C      NTK1=NTRK(I,I)
C      CALL RDDISK(NTK1,AKW1)
C      EXAMINE THE B.C. CODES FOR NODE NBC2 AND PERFORM THE
C      NECESSARY MODIFICATIONS TO THE DIAGONAL BLOCK AND THE
C      RIGHT HAND VECTOR
C
205      I61=0
C      I6=0
C      DO 250 K=1,NDF
C      K1=K+1
C      M=NBC(K1)+1
C      I1=NBC(I1)-(I-1)*NS+K
C      GO TO (250,230,220),M
C      IF (I6.NE.0) GO TO 222
220      READ(22) BCON
C      I5=I5+1
C      I6=1
C      I61=1
C      DO 225 L=1,NS
C      RB1(L)=RB1(L)-AK1(L,I1)*BCON(K)
222      I4=I4+1
225      BCON1(I4,1)=I1
C      BCON1(I4,2)=BCON(K)
C      AK1(I1,I1)=AK1(I1,I1)+ABIGN
230      CONTINUE
250      J3=MM+1
C      MML=I/MMF-(I-MMF)/MMF
C      MM1=MM-(I-MMF)*MML
C      J4=MM1
C
C      IF THE B.C. APPLIED IS THE LAST ONE, NO FURTHER REDUCTION
C      OF ROW 'I' IS REQUIRED
C      PROCEED WITH REDUCTION OF BLOCKS SYMMETRIC TO THE ONES OF
C      ROW 'I'
C
C      IF(NBC2.EQ.NBCH) GO TO 410
C      READ THE NEXT B.C.
C
C      READ(15) NBC
C      NBC1=NBC(I1)+NDF
C      NBC2=NBC1/NDF
C      I1=I1+1
C
C      IF THE NEW B.C. DOES NOT CORRESPOND TO THE DIAGONAL BLOCK

```



```

BC002260
BC002270
BC002280
BC002290
BC002300
BC002310
BC002320
BC002330
BC002340
BC002350
BC002360
BC002370
BC002380
BC002390
BC002400
BC002410
BC002420
BC002430
BC002440
BC002450
BC002460
BC002470
BC002480
BC002490
BC002500
BC002510
BC002520
BC002530
BC002540
BC002550
BC002560
BC002570
BC002580
BC002590
BC002600
BC002610
BC002620
BC002630
BC002640
BC002650
BC002660
BC002670
BC002680
BC002690
BC002700
BC002710
BC002720
BC002730

PROCEED WITH REDUCTION OF NON DIAGONAL BLOCKS AND THEIR
SYMMETRICS
IF(NBC1.LE.IH) GO TO 205
*****
AFTER COMPLETION OF REDUCTION OF DIAGONAL BLOCK THE
FOLLOWING STEPS ARE EXECUTED
(1) DETERMINATION OF BLOCK 'J1' CORRESPONDING TO THE
LAST B.C. READ FROM DATA
(2) IF BLOCK 'J1' IS THE NEXT TO THE DIAGONAL, CONTROL
IS TRANSFERRED TO THE SEGMENT REDUCING THE RIGHT HAND
VECTORS CORRESPONDING TO NON DIAGONAL BLOCKS AND
THEIR SYMMETRICS
(3) IF STEP (2) IS NOT EXECUTED REDUCE THE RIGHT HAND
VECTORS CORRESPONDING TO BLOCKS SYMMETRIC TO THE
ONES BETWEEN THE DIAGONAL 2ND 'J1'
*****
STEP (1)
400 J1=(NBC1+NS-1)/NS
J2=J1+1
J3=J1-I+1
IFR=J1
IF(I61.EQ.0) GO TO 490
IH=J1*NS
*****
STEP (2)
IF(J1.EQ.J2) GO TO 490
*****
STEP (3)
J4=J3-1
IF(J4.GT.MM) J4=MM
IF(I.EQ.NN) GO TO 950
DO 450 J=2,J4
NTK2=NTRK(I,J)
CALL RDISK(NTK2,AKW2)
NTKR2=J+I-1
CALL RDSKR2(NTKR2)
DO 430 L1=1,I4
IR=BCON1(L1,I)
DO 430 L2=1,NS
RB2(L2)=RB2(L2)-AK2(IR,L2)*BCON1(L1,2)
430

```



```

C 450 CALL WDSKR2(NTKR2)
C *****
C 490 CALL WRDISK(NTK1,AKW1)
500 IF(J3.GT.MM) GO TO 850
    MML=I/MMF-(I-MMF)/MMF
    MML=MM-(I-MMF)*MML
    DO 800 J=J3,MM1
    IH=(J+(I-1))*NS
        READ BLOCK (I,J)
    NTK2=NTKR(I,J)
    CALL RDISK(NTK2,AKW2)
    IHL=(J+I-2)*NS+1
    IPD=(IH+1-NBC1)*(IHL-NBC1)
    IF THE B.C. NBC2 DOES NOT CORRESPOND TO BLOCK (I,J)
    PROCEED WITH REDUCTION OF ITS SYMMETRIC BLOCK
        IF(IPD.GT.0) GO TO 600
        MODIFY RIGHT HAND VECTOR 'I' FOR ALL B.C.'S CORRESPONDING
        TO BLOCK (I,J)
510 I6=0
    DO 560 K=1,NDF
    K1=K+1
    IF(NBC(K1).NE.2) GO TO 560
    IF(I6.NE.0) GO TO 550
    READ(22) BCON
    I6=1
    II=NBC(1)+K-(I+J-2)*NS
    DO 555 L=1,NS
    RB1(L)=RB1(L)-AK2(L,II)*BCON(K)
550 CONTINUE
550 IF THE B.C. APPLIED IS THE LAST ONE, NO FURTHER REDUCTION
    IN ROW 'I' IS REQUIRED
    PROCEED WITH REDUCTION OF ITS SYMMETRIC BLOCK
        IF(NBC2.EQ.NBCH) GO TO 600
    READ(15) NBC
    NBC1=NBC(1)+NDF
    NBC2=NBC1/NDF
        IF THE NEW B.C. DOES NOT CORRESPOND TO BLOCK (I,J) PROCEED

```

```

BC002740
BC002750
BC002760
BC002770
BC002780
BC002790
BC002800
BC002810
BC002820
BC002830
BC002840
BC002850
BC002860
BC002870
BC002880
BC002890
BC002900
BC002910
BC002920
BC002930
BC002940
BC002950
BC002960
BC002970
BC002980
BC002990
BC003000
BC003010
BC003020
BC003030
BC003040
BC003050
BC003060
BC003070
BC003080
BC003090
BC003100
BC003110
BC003120
BC003130
BC003140
BC003150
BC003160
BC003170
BC003180
BC003190
BC003200
BC003210

```













```

CALL SYMINV(AK2,NS,RB1,RB2,IFLG)
      (1B) IF A(N,1) IS SINGULAR GIVE ERROR MESSAGE AND
            STOP EXECUTION FOR THIS PROBLEM
            IF A(N,1) IS NEARLY SINGULAR GIVE WARNING
            MESSAGE AND CONTINUE EXECUTION
IF(IFLG.EQ.1) CALL ERROR4(N)
IF(IFLG.EQ.2) CALL ERROR5(N)
      (1C) READ B(N)
            MULTIPLY B(N) BY A*(N,1)
            STORE THE PRODUCT IN PLACE OF B(N)
NTR=N
CALL RDSKR1(NTR)
CALL MULT(AK2,RB1,RB2,NS,NS,1)
CALL WDSKR2(NTR)
      (1D) IF ROW 'N' IS THE LAST ROW SKIP THE FOLLOWING
            STEPS AND START BACK SUBSTITUTION
IF(N.EQ.NN) GO TO 300
      (2) REDUCTION OF NON DIAGONAL COEFFICIENT BLOCKS OF
            ROW 'N'
            IF BLANK BLOCKS EXIST SKIP THEM
IF(N.GT.(NN-MM+1)) KMM=KMM-1
            THE FOLLOWING STEPS ARE REPEATED FOR ALL BLOCKS A(N,K
            (2A) READ A(N,K)
                    MULTIPLY A(N,K) BY A*(N,1)
                    STORE THE PRODUCT IN THE PLACE OF A(N,K)
DO 200 K=2,KMM
NTR=NTRK(N,K)
CALL RDDISK(NTRK,AK1)
CALL MULT(AK2,AK1,AK3,NS,NS,NS)
CALL WRDISK(NTRK,AK3)

```

CCCCCCCC CCCCCCCC CCCCCCCCCCCCCC



SLV01060  
SLV01070  
SLV01080  
SLV01090  
SLV01100  
SLV01110  
SLV01120  
SLV01130  
SLV01140  
SLV01150  
SLV01160  
SLV01170  
SLV01180  
SLV01190  
SLV01200  
SLV01210  
SLV01220  
SLV01230  
SLV01240  
SLV01250  
SLV01260  
SLV01270  
SLV01280  
SLV01290  
SLV01300  
SLV01310  
SLV01320  
SLV01330  
SLV01340  
SLV01350  
SLV01360  
SLV01370  
SLV01380  
SLV01390  
SLV01400  
SLV01410  
SLV01420  
SLV01430  
SLV01440  
SLV01450  
SLV01460  
SLV01470  
SLV01480  
SLV01490  
SLV01500  
SLV01510  
SLV01520  
SLV01530

```

(2B) COMPUTE A'(N,K)
      STORE A'NN,K) IN THE BLANK BLOCK A(NN,K)

DO 150 I=1,NS
  II=(I-1)*NS
  DO 150 J=1,NS
    IL=II+J
    IR=I+(J-1)*NS
    AK3(IL)=AK1(IR)
    NTK=NTRK(NN,K)
    CALL WRDISK(NTK,AK3)
  CONTINUE
150
200

(3) REDUCE THE REMAINING ROWS STARTING WITH ROW 'N+1'
    THE FOLLOWING STEPS ARE REPEATED FOR EVERY ROW 'I'

DO 260 L=2,KMM
  I=N+L-1
  IF(I.GT.NN) GO TO 260
  J=0

(2A) READ A'(N,L) STORED IN BLOCK (NN,L)

NTK=NTRK(NN,L)
CALL RDDISK (NTK,AK2)

(2B) THE FOLLOWING STEPS ARE REPEATED FOR ALL BLOCKS
      OF ROW 'I'
      READ A(N,K)
      MULTIPLY A(N,K) BY A'(N,L)
      READ A(I,J) AND SUBTRACT FROM IT THE PRODUCT
      OF THE PREVIOUS STEP
      STORE THE RESULT IN THE PLACE OF A(I,J)

DO 250 K=L,KMM
  J=J+1
  NTK=NTRK(N,K)
  CALL RDDISK(NTK,AK1)
  CALL MULT(AK2,AK1,AK3,NS,NS,NS)

```





```

NTK=NTRK(I,J)
CALL RDDISK(NTK,AK1)
DO 210 I=1,NCOUNT
210 AK1(I1)=AK1(I1)-AK3(I1)
CALL WRDISK(NTK,AK1)
250 CONTINUE

      (2C) MULTIPLY B(N) BY A'(N,L)
      READ B(I)
      FORM B(I)-B(N)*A'(N,L)
      STORE THE PRODUCT IN THE PLACE OF B(I)

      CALL MULT(AK2,RB2,RB3,NS,NS,1)
      NTR=I
      CALL RDSKR1(NTR)
      DO 255 I=1,NS
255 RB1(I1)=RB1(I1)-RB3(I1)
      CALL WDSKR1(NTR)
260 CONTINUE
      GO TO 100

*****
      BACK SUBSTITUTION
      THE FOLLOWING STEPS ARE REPEATED FOR ALL ROWS STARTING
      WITH ROW 'NN-1'

300 N=N-1
      IF THE LAST ROW REDUCED WAS ROW NO 1 RETURN
      IF(N.EQ.0) GO TO 500
      READ B(N)
      NT1=N
      CALL RDSKR3(NT1)
      THE FOLLOWING ARE REPEATED FOR ALL BLOCKS A(N,K)
      (1) READ A(N,K)
      (2) READ B(L) WHERE B(L)= VECTOR OF SOLUTIONS FOR THE
          UNKNOWNNS CORRESPONDING TO BLOCK A(N,K) FOUND IN

```

SLV01540  
 SLV01550  
 SLV01560  
 SLV01570  
 SLV01580  
 SLV01590  
 SLV01600  
 SLV01610  
 SLV01620  
 SLV01630  
 SLV01640  
 SLV01650  
 SLV01660  
 SLV01670  
 SLV01680  
 SLV01690  
 SLV01700  
 SLV01710  
 SLV01720  
 SLV01730  
 SLV01740  
 SLV01750  
 SLV01760  
 SLV01770  
 SLV01780  
 SLV01790  
 SLV01800  
 SLV01810  
 SLV01820  
 SLV01830  
 SLV01840  
 SLV01850  
 SLV01860  
 SLV01870  
 SLV01880  
 SLV01890  
 SLV01900  
 SLV01910  
 SLV01920  
 SLV01930  
 SLV01940  
 SLV01950  
 SLV01960  
 SLV01970  
 SLV01980  
 SLV01990  
 SLV02000  
 SLV02010



PREVIOUS STEPS OF BACK SUBSTITUTION

(3) FORM  $B(NE-A(N,K)*B(L))$

AT THE END OF THE OPERATIONS FOR ALL BLOCKS OF ROW 'N',  
THE VECTOR B(N) CONTAINS THE SOLUTIONS FOR THE UNKNOWNNS  
CORRESPONDING TO THE DIAGONAL BLOCK A(N,1) AND IS STORED  
ON DISK

```

DO 400 K=2,KMM
L=N+K-1
IF(L.GT.NN) GO TO 400
NTK=NTRK(N,K)
CALL RDDISK(NTK,AK1)
NTR=L
CALL RDSKRI(NTR)
CALL MULT(AK1,RB1,RB2,NS,NS,1)
DO 310 K1=1,NS
RB3(K1)=RB3(K1)-RB2(K1)
CONTINUE
CALL WDSKR3(NT1)
KMM=KMM+1
IF(KMM.GT.MM) KMM=MM
GO TO 300
RETURN
END

```

310  
400

500

SUBROUTINE DISP

```

IMPLICIT REAL*8(A-H,O-Z)
COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
1NPBC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDFP,KLINE,NREC7,KEL
COMMON /NB2/ NCON(20),NRC(4)
COMMON /B3/ AK1(60,60),AK2(60,60),RB1(60),RB2(60),
1RB3(60)
DIMENSION REACT(4),REACT1(3)
EQUIVALENCE (REACT(1),RB2(1)),(REACT(2),REACT1(1))

```

```

ABIGN=1.0D20
ZRO=0.0D0
REWIND 15
REWIND 17
READ(15) NRC
NRC1=NRC(1)+NDF
INDEX=1
NPBL=NS/NDF

```

SLV02020  
SLV02030  
SLV02040  
SLV02050  
SLV02060  
SLV02070  
SLV02080  
SLV02090  
SLV02100  
SLV02110  
SLV02120  
SLV02130  
SLV02140  
SLV02150  
SLV02160  
SLV02170  
SLV02180  
SLV02190  
SLV02200  
SLV02210  
SLV02220  
SLV02230  
SLV02240  
SLV02250  
SLV02260  
SLV02270  
DSP00010

DSP00020  
DSP00030  
DSP00040  
DSP00050  
DSP00060  
DSP00070  
DSP00080  
DSP00090  
DSP00100  
DSP00110  
DSP00120  
DSP00130  
DSP00140  
DSP00150  
DSP00160  
DSP00170  
DSP00180  
DSP00190  
DSP00200



```

C      JLL=NDPT-(NDPT/NPBL)*NPBL
C      IF(JLL.EQ.0) JLL=NPBL
C      NLI=0
C      WRITE(6,1000)
C
C      TRANSFER OF RIGHT HAND BLOCKS IN SEQUENCE FROM DISK INTO
C      CORE
C
C      DO 280 I=1,NN
C      CALL RDSKRI(I)
C      CALL WDISKB(I)
C
C      IF THE CONSTRAINED POINT EXAMINED IS NOT INCLUDED IN THE
C      RIGHT HAND BLOCK 'I', SKIP NEXT SEGMENT
C
C      IM1=I-1
C      ICTI=IM1*NPBL
C      IH=I*NS
C      IL=IM1*NS+1
C      ICHECK=(NBC1-IH)*(IL-NBC1)
C      IF(ICHECK.LT.0) GO TO 255
C
C      DETERMINE THE REACTIONS AT THE CONSTRAINED POINTS AND
C      MODIFY CORRESPONDINGLY THE RIGHT HAND VECTOR
C
C      DO 215 J=1,NDFP
C      REACT(J)=ZRO
C      I1=NBC(I)-NS*(I-1)+1
C      I2=I1+1
C      I3=I2+1
C
C      IF(NBC(2).EQ.0) GO TO 220
C      REACT(2)=RBI(I1)*ABIGN
C      RBI(I1)=ZRO
C      IF(NBC(3).EQ.0) GO TO 240
C      REACT(3)=RBI(I2)*ABIGN
C      RBI(I2)=ZRO
C      IF(NBC(4).EQ.0) GO TO 250
C      REACT(4)=RBI(I3)*ABIGN
C      RBI(I3)=ZRO
C      ANBC=NBC(1)/NDF+1
C      REACT(1)=ANBC
C      WRITE(17) REACT
C      IF(INDEX.EQ.NPBC) GO TO 255
C      READ(15) NBC
C      NBC1=NBC(1)+NDF
C      INDEX=INDEX+1

```

DSP00210  
 DSP00220  
 DSP00230  
 DSP00240  
 DSP00250  
 DSP00260  
 DSP00270  
 DSP00280  
 DSP00290  
 DSP00300  
 DSP00310  
 DSP00320  
 DSP00330  
 DSP00340  
 DSP00350  
 DSP00360  
 DSP00370  
 DSP00380  
 DSP00390  
 DSP00400  
 DSP00410  
 DSP00420  
 DSP00430  
 DSP00440  
 DSP00450  
 DSP00460  
 DSP00470  
 DSP00480  
 DSP00490  
 DSP00500  
 DSP00510  
 DSP00520  
 DSP00530  
 DSP00540  
 DSP00550  
 DSP00560  
 DSP00570  
 DSP00580  
 DSP00590  
 DSP00600  
 DSP00610  
 DSP00620  
 DSP00630  
 DSP00640  
 DSP00650  
 DSP00660  
 DSP00670  
 DSP00680



DSP00690  
 DSP00700  
 DSP00710  
 DSP00720  
 DSP00730  
 DSP00740  
 DSP00750  
 DSP00760  
 DSP00770  
 DSP00780  
 DSP00790  
 DSP00800  
 DSP00810  
 DSP00820  
 DSP00830  
 DSP00840  
 DSP00850  
 DSP00860  
 DSP00870  
 DSP00880  
 DSP00890  
 DSP00900  
 DSP00910  
 DSP00920  
 DSP00930  
 DSP00940  
 DSP00950  
 DSP00960  
 DSP00970  
 DSP00980  
 DSP00990  
 DSP01000  
 DSP01010  
 DSP01020  
 DSP01030  
 DSP01040  
 DSP01050  
 DSP01060  
 DSP01070  
 DSP01080  
 DSP01090  
 DSP01100  
 DSP01110  
 DSP01120

```

C 255 GO TO 210
      NL2=NL1+NPBL
      JL=NPBL
      IF(I.EQ.NN) JL=JLL
      IF(NL2.GT.NLINE) GO TO 262
      DO 260 M=1,JL
      ICT=ICTI+M
      K1=(M-1)*NDF+1
      K2=K1+2
      WRITE(6,2000) ICT,(RB1(N1),N1=K1,K2)
      NL1=NL2
      GO TO 280
262 J2=NLINE-NL1
      DO 264 N=1,J2
      ICT=ICTI+N
      K1=(N-1)*NDF+1
      K2=K1+2
      WRITE(6,2000) ICT,(RB1(N2),N2=K1,K2)
      NL1=NPBL-J2
      WRITE(6,1000)
      J1=J2+1
      DO 266 M1=J1,JL
      ICT=ICTI+M1
      K1=(M1-1)*NDF+1
      K2=K1+2
      WRITE(6,2000) ICT,(RB1(N3),N3=K1,K2)
266 CONTINUE
280
C 285 SUMX=ZRO
      SUMY=ZRO
      SUMZ=ZRO
      I2=ZRO
      REWIND 17
      PRINT THE REACTIONS
C
C
290 I1=1
      I2=I2+NLINE
      IF(NPBC.LE.I2) I2=NPBC
      WRITE(6,1100)
      DO 300 L3=I1,I2
      READ(17) REACT
      ICT=REACT(I)

```









STR00230  
STR00240  
STR00250  
STR00255  
STR00260  
STR00270  
STR00280  
STR00290  
STR00300  
STR00310  
STR00320  
STR00330  
STR00340  
STR00350  
STR00360  
STR00370  
STR00380  
STR00390  
STR00400  
STR00410  
STR00420  
STR00430  
STR00440  
STR00450  
STR00460  
STR00470  
STR00480  
STR00490  
STR00500  
STR00510  
STR00520  
STR00530  
STR00540  
STR00550  
STR00560  
STR00570  
STR00580  
STR00590  
STR00600  
STR00610  
STR00620  
STR00630  
STR00640  
STR00650  
STR00660  
STR00670  
STR00680  
STR00690

```

C      6-1.000,-1.000/
      READ(9) NCON
      IF(KEL.EQ.0) GO TO 200
      WRITE(6,1000) I
C      FORM THE VECTOR OF ELEMENT NODAL DISPLACEMENTS
C      200 DO 300 J=1,NPEL
          LM(J)=NCON(J)
          JJ=LM(J)
          JEL=3*(J-1)
          DO 250 K=1,NDF
              CALL RDISKB(JJ)
              J2=JEL+K
              UEL(J2)=UJNT(K)
          CONTINUE
C      250
C      300
C      COMPUTATION OF ELEMENT NODAL STRAINS
C      DO 400 J=1,NPEL
          JJ=NCON(J)
          X=CORDG(J,1)
          Y=CORDG(J,2)
          Z=CORDG(J,3)
          CALL FORMK(X,Y,Z,2)
          DO 310 J1=1,6
              SSSS(J1)=0.000
              SSNN(J1)=0.000
              DO 310 J2=1,NST
                  SSNN(J1)=SSNN(J1)+B(J1,J2)*UEL(J2)
          CONTINUE
C      310
C      COMPUTATION OF NODAL STRESSES FROM NODAL STRAINS
C      DO 320 J1=1,3
          DO 320 J2=1,3
              SSSS(J1)=SSSS(J1)+SSNN(J2)*ELAST(J1,J2)
          DO 330 K1=4,6
              SSSS(K1)=SSSS(K1)+SSNN(K1)*ELAST(K1,K1)
          CONTINUE
C      320
C      330
C      FORMATION OF NODAL STRESS AND STRAIN VECTORS FOR THIS
C      ELEMENT
C      CALL RDISKS(JJ)
C      CALL RDISKN(JJ)
C      SSSS1(7)=SSSS1(7)+1.000
C      SSNN1(7)=SSNN1(7)+1.000
C      DO 350 K=1,6

```







```

C      EQUIVALENCE (STK1(1),AK1(1,1)),(STK2(1),AK1(1,16)),
C      1 (STK3(1),AK1(1,31)),(STK4(1),AK1(1,46))
C
C      NCOUNT=NS*NS
C      LREC7=NCOUNT/NREC7
C      NS1=NS/NDF
C      RETURN
C
C      ENTRY WRDISK(NTRACK,A)
C
C      I7=(NTRACK-1)*NREC7+1
C      N1=1
C      DO 10 L=1,NREC7
C      N2=L*LREC7
C      WRITE(7,I7)
C      10 N1=N2+1
C      RETURN
C
C      ENTRY RDDISK(NTRACK,A)
C
C      I7=(NTRACK-1)*NREC7+1
C      N1=1
C      DO 20 L=1,NREC7
C      N2=L*LREC7
C      READ(7,I7,ERR=500)
C      20 N1=N2+1
C      RETURN
C
C      ENTRY WDISK1(NTRACK)
C
C      I8=(NTRACK-1)*4+1
C      WRITE(8,I8) STK1
C      WRITE(8,I8) STK2
C      WRITE(8,I8) STK3
C      WRITE(8,I8) STK4
C      RETURN
C
C      ENTRY RDISK1(NTRACK,A)
C
C      120 DSK00130
C      130 DSK00130
C      140 DSK00140
C      150 DSK00150
C      160 DSK00160
C      170 DSK00170
C      180 DSK00180
C      190 DSK00190
C      200 DSK00200
C      210 DSK00210
C      220 DSK00220
C      230 DSK00230
C      240 DSK00240
C      250 DSK00250
C      260 DSK00260
C      270 DSK00270
C      280 DSK00280
C      290 DSK00290
C      300 DSK00300
C      310 DSK00310
C      320 DSK00320
C      330 DSK00330
C      340 DSK00340
C      350 DSK00350
C      360 DSK00360
C      370 DSK00370
C      380 DSK00380
C      390 DSK00390
C      400 DSK00400
C      410 DSK00410
C      420 DSK00420
C      430 DSK00430
C      440 DSK00440
C      450 DSK00450
C      460 DSK00460
C      470 DSK00470
C      480 DSK00480
C      490 DSK00490
C      500 DSK00500
C      510 DSK00510
C      520 DSK00520
C      530 DSK00530
C      540 DSK00540
C      550 DSK00550
C      560 DSK00560
C      570 DSK00570
C      580 DSK00580
C      590 DSK00590
C      600 DSK00600
C      610 DSK00610
C      620 DSK00620
C      630 DSK00630

```

















IN 'DEFINE FILE', STATEMENTS AND CONTROL CARDS EXCEPT FOR  
 THE NUMBER OF RECORDS OF FILE NO 7 (SEE 'ERROR2')  
 THE NUMBER OF DIFFERENT MATERIALS IS ALSO CHECKED

ERR00220  
 ERR00230  
 ERR00240  
 ERR00250  
 ERR00260  
 ERR00270  
 ERR00280  
 ERR00290  
 ERR00300  
 ERR00310  
 ERR00320  
 ERR00330  
 ERR00340  
 ERR00350  
 ERR00360  
 ERR00370  
 ERR00380  
 ERR00390  
 ERR00400  
 ERR00410  
 ERR00420  
 ERR00430  
 ERR00440  
 ERR00450  
 ERR00460  
 ERR00470  
 ERR00480  
 ERR00490  
 ERR00500  
 ERR00510  
 ERR00520  
 ERR00530  
 ERR00540  
 ERR00550  
 ERR00560  
 ERR00570  
 ERR00580  
 ERR00590  
 ERR00600  
 ERR00610  
 ERR00620  
 ERR00630  
 ERR00640  
 ERR00650  
 ERR00660  
 ERR00670  
 ERR00680  
 ERR00690

ISTOP=0  
 ISTOP9=0  
 ISTOP16=0

FILE NO 7 (EXCEPT NR7)

IFILE=7  
 IF(LRB7-NCOUNT\*8/NREC7) 40,60,50

40 WRITE(6,2000)  
 WRITE(6,8000) IFILE

ISTOP=1  
 GO TO 60

50 WRITE(6,3000)  
 WRITE(6,8500) IFILE

60 IF(LRW7-NCOUNT\*2/NREC7) 70,90,80

70 WRITE(6,2000)  
 WRITE(6,9000) IFILE

ISTOP=1  
 GO TO 80

80 WRITE(6,3000)  
 WRITE(6,9500) IFILE

90 IF((LRW7\*4-LRB7).EQ.0) GO TO 95

WRITE(6,2000)  
 WRITE(6,9700) IFILE

ISTOP=1

FILE NO 8

95 IFILE=8  
 IF(NR8-NEL\*4) 100,120,110

100 WRITE(6,2000)  
 WRITE(6,6000) IFILE

ISTOP=1  
 GO TO 120

110 WRITE(6,3000)  
 WRITE(6,5000) IFILE

120 IF(LRB8-7200) 130,150,140

130 WRITE(6,2000)  
 WRITE(6,8000) IFILE

ISTOP=1  
 GO TO 150





ERR00700  
ERR00710  
ERR00720  
ERR00730  
ERR00740  
ERR00750  
ERR00760  
ERR00770  
ERR00780  
ERR00790  
ERR00800  
ERR00810  
ERR00820  
ERR00830  
ERR00840  
ERR00850  
ERR00860  
ERR00870  
ERR00880  
ERR00890  
ERR00900  
ERR00910  
ERR00920  
ERR00930  
ERR00940  
ERR00950  
ERR00960  
ERR00970  
ERR00980  
ERR00990  
ERR01000  
ERR01010  
ERR01020  
ERR01030  
ERR01040  
ERR01050  
ERR01060  
ERR01070  
ERR01080  
ERR01090  
ERR01100  
ERR01110  
ERR01120  
ERR01130  
ERR01140  
ERR01150  
ERR01160  
ERR01170

```

140 WRITE(6,3000)      IFILE
    C  WRITE(6,8500)
150 IF(LRW8-1800) 160,180,170
160 WRITE(6,2000)  IFILE
    WRITE(6,9000)
    ISTOP=1
    GO TO 180
170 WRITE(6,3000)      IFILE
    C  WRITE(6,9500)
180 IF((LRW8*4-LRB8).EQ.0) GO TO 185
    WRITE(6,2000)  IFILE
    ISTOP=1
    C  C
    C  FILE NO 9
185 IFILE=9
    IF(NR9-NEL) 190,210,200
190 WRITE(6,6000)  IFILE
    ISTOP=1
    GO TO 210
200 WRITE(6,3000)      IFILE
    C  WRITE(6,5000)
210 IF(LRB9-80) 220,240,230
220 WRITE(6,2000)  IFILE
    ISTOP=1
    GO TO 240
230 WRITE(6,3000)      IFILE
    C  WRITE(6,8500)
    C  C
    C  FILE NO 10
240 IFILE=10
    IF(NR10-NDPT) 250,270,260
250 WRITE(6,2000)  IFILE
    WRITE(6,6000)
    ISTOP=1
    GO TO 270
260 WRITE(6,3000)      IFILE
    C  WRITE(6,5000)
270 IF(LRB10-24) 280,300,290
280 WRITE(6,2000)
    C

```



ERROR1180  
 ERROR1190  
 ERROR1200  
 ERROR1210  
 ERROR1220  
 ERROR1230  
 ERROR1240  
 ERROR1250  
 ERROR1260  
 ERROR1270  
 ERROR1280  
 ERROR1290  
 ERROR1300  
 ERROR1310  
 ERROR1320  
 ERROR1330  
 ERROR1340  
 ERROR1350  
 ERROR1360  
 ERROR1370  
 ERROR1380  
 ERROR1390  
 ERROR1400  
 ERROR1410  
 ERROR1420  
 ERROR1430  
 ERROR1440  
 ERROR1450  
 ERROR1460  
 ERROR1470  
 ERROR1480  
 ERROR1490  
 ERROR1500  
 ERROR1510  
 ERROR1520  
 ERROR1530  
 ERROR1540  
 ERROR1550  
 ERROR1560  
 ERROR1570  
 ERROR1580  
 ERROR1590  
 ERROR1600  
 ERROR1610  
 ERROR1620  
 ERROR1630  
 ERROR1640  
 ERROR1650

```

WRITE(6,8000) IFILE
I$TOP=1
GO TO 300
290 WRITE(6,3000)
WRITE(6,8500) IFILE

C 300 IF(LRW10-6) 310,330,320
310 WRITE(6,2000)
WRITE(6,9000) IFILE
I$TOP=1
GO TO 330
320 WRITE(6,3000)
WRITE(6,9500) IFILE

C 330 IF((LRW10*4-LRB10).EQ.0) GO TO 335
WRITE(6,2000)
WRITE(6,9700) IFILE
I$TOP=1

C
C FILE NO 12
C
335 IFILE=12
IF(NR12-NDPT) 340,360,350
340 WRITE(6,2000)
WRITE(6,6000) IFILE
I$TOP=1
GO TO 360
350 WRITE(6,3000)
WRITE(6,5000) IFILE

C 360 IF(LPB12-56) 370,390,380
370 WRITE(6,2000)
WRITE(6,8000) IFILE
I$TOP=1
GO TO 390
380 WRITE(6,3000)
WRITE(6,8500) IFILE

C 390 IF(LRW12-14) 400,420,410
400 WRITE(6,2000)
WRITE(6,9000) IFILE
I$TOP=1
GO TO 420
410 WRITE(6,3000)
WRITE(6,9500) IFILE

C 420 IF((LRW12*4-LRB12).EQ.0) GO TO 425
WRITE(6,2000)

```



```

ERR01660
ERR01670
ERR01680
ERR01690
ERR01700
ERR01710
ERR01720
ERR01730
ERR01740
ERR01750
ERR01760
ERR01770
ERR01780
ERR01790
ERR01800
ERR01810
ERR01820
ERR01830
ERR01840
ERR01850
ERR01860
ERR01870
ERR01880
ERR01890
ERR01900
ERR01910
ERR01920
ERR01930
ERR01940
ERR01950
ERR01960
ERR01970
ERR01980
ERR01990
ERR02000
ERR02010
ERR02020
ERR02030
ERR02040
ERR02050
ERR02060
ERR02070
ERR02080
ERR02090
ERR02100
ERR02110
ERR02120
ERR02130

```

```

C      WRITE(6,9700) IFILE
C      ISTOP=1
C      FILE NO 13
425  IFILE=13
430  IF(NR13-NDPT) 430,450,440
      WRITE(6,2000) IFILE
      ISTOP=1
      GO TO 450
440  WRITE(6,3000)
      WRITE(6,5000) IFILE
C
450  IF(LRB13-56) 460,480,470
460  WRITE(6,2000)
      WRITE(6,8000) IFILE
      ISTOP=1
      GO TO 480
470  WRITE(6,3000)
      WRITE(6,8500) IFILE
C
480  IF(LRW13-14) 490,510,500
490  WRITE(6,2000)
      WRITE(6,9000) IFILE
      ISTOP=1
      GO TO 510
500  WRITE(6,3000)
      WRITE(6,9500) IFILE
C
510  IF((LRW13*4-LRB13).EQ.0) GO TO 515
      WRITE(6,2000)
      WRITE(6,9700) IFILE
      ISTOP=1
C      FILE NO 14
C
515  IFILE=14
520  IF(NR14-NCLD) 520,540,530
      WRITE(6,2000)
      WRITE(6,6000) IFILE
      ISTOP=1
      GO TO 540
530  WRITE(6,3000)
      WRITE(6,5000) IFILE
C
540  IF(LRB14-32) 550,570,560
550  WRITE(6,2000)

```



```

WRITE(6,8000) IFILE
ISTOP=1
GO TO 570
560 WRITE(6,3000)
WRITE(6,8500) IFILE
C
C
C
FILE NO 15
570 IFILE=15
IF(NR15-NPBC) 580,600,590
580 WRITE(6,2000)
WRITE(6,6000) IFILE
ISTOP=1
GO TO 600
590 WRITE(6,3000)
WRITE(6,5000) IFILE
C
600 IF(LRB15-16) 610,630,620
610 WRITE(6,2000)
WRITE(6,8000) IFILE
ISTOP=1
GO TO 630
620 WRITE(6,3000)
WRITE(6,8500) IFILE
C
C
C
FILE NO 16
630 IF(NMAT.EQ.1) GO TO 690
IFILE=16
IF(NR16-NEL) 640,660,650
640 WRITE(6,2000)
WRITE(6,6000) IFILE
ISTOP=1
GO TO 660
650 WRITE(6,3000)
WRITE(6,5000) IFILE
C
660 IF(LRB16-4) 670,690,680
670 WRITE(6,2000)
WRITE(6,8000) IFILE
ISTOP=1
GO TO 690
680 WRITE(6,3000)
WRITE(6,8500) IFILE
C
C
C
FILE NO 17
690 IFILE=17

```

```

ERR02140
ERR02150
ERR02160
ERR02170
ERR02180
ERR02190
ERR02200
ERR02210
ERR02220
ERR02230
ERR02240
ERR02250
ERR02260
ERR02270
ERR02280
ERR02290
ERR02300
ERR02310
ERR02320
ERR02330
ERR02340
ERR02350
ERR02360
ERR02370
ERR02380
ERR02390
ERR02400
ERR02410
ERR02420
ERR02430
ERR02440
ERR02450
ERR02460
ERR02470
ERR02480
ERR02490
ERR02500
ERR02510
ERR02520
ERR02530
ERR02540
ERR02550
ERR02560
ERR02570
ERR02580
ERR02590
ERR02600
ERR02610

```





```

700 IF(NR17-NPBC) 700,720,710
      WRITE(6,2000)
      WRITE(6,6000) IFILE
      ISTOP=1
      GO TO 720
710 WRITE(6,3000)
      WRITE(6,5000) IFILE
      C
720 IF(LRB17-32) 730,750,740
730 WRITE(6,2000)
      WRITE(6,8000) IFILE
      ISTOP=1
      GO TO 750
740 WRITE(6,3000)
      WRITE(6,8500) IFILE
      C
      C
      C
      FILE NO 19
750 IFILE=19
      IF(NR19-NEL) 760,780,770
760 WRITE(6,2000)
      WRITE(6,6000) IFILE
      ISTOP=1
      GO TO 780
770 WRITE(6,3000)
      WRITE(6,5000) IFILE
      C
780 IF(LRB19-480) 790,810,800
790 WRITE(6,2000)
      WRITE(6,8000) IFILE
      ISTOP=1
      GO TO 810
800 WRITE(6,3000)
      WRITE(6,8500) IFILE
      C
      C
      C
      FILE NO 20
810 IFILE=20
      IF(NR20-NN) 820,840,830
820 WRITE(6,2000)
      WRITE(6,6000) IFILE
      ISTOP=1
      GO TO 840
830 WRITE(6,3000)
      WRITE(6,5000) IFILE
      C
840 IF(LPB20-NS*8) 850,870,860
850 WRITE(6,2000)

```

```

ERR02620
ERR02630
ERR02640
ERR02650
ERR02660
ERR02670
ERR02680
ERR02690
ERR02700
ERR02710
ERR02720
ERR02730
ERR02740
ERR02750
ERR02760
ERR02770
ERR02780
ERR02790
ERR02800
ERR02810
ERR02820
ERR02830
ERR02840
ERR02850
ERR02860
ERR02870
ERR02880
ERR02890
ERR02900
ERR02910
ERR02920
ERR02930
ERR02940
ERR02950
ERR02960
ERR02970
ERR02980
ERR02990
ERR03000
ERR03010
ERR03020
ERR03030
ERR03040
ERR03050
ERR03060
ERR03070
ERR03080
ERR03090

```



```

ERR031100
ERR031110
ERR031120
ERR031130
ERR031140
ERR031150
ERR031160
ERR031170
ERR031180
ERR031190
ERR032000
ERR032100
ERR032200
ERR032300
ERR032400
ERR032500
ERR032600
ERR032700
ERR032800
ERR032900
ERR033000
ERR033100
ERR033200
ERR033300
ERR033400
ERR033500
ERR033600
ERR033700
ERR033800
ERR033900
ERR034000
ERR034100
ERR034200
ERR034300
ERR034400
ERR034500
ERR034600
ERR034700
ERR034800
ERR034900
ERR035000
ERR035100
ERR035200
ERR035300
ERR035400
ERR035500
ERR035600
ERR035700

```

```

C
      WRITE(6,8000) IFIL
      ISTOP=1
      GO TO 870
860  WRITE(6,3000)
      WRITE(6,8500) IFIL
C
870  IF(LRW20-NS*2) 880,900,890
880  WRITE(6,2000)
      WRITE(6,9000) IFIL
      ISTOP=1
      GO TO 900
890  WRITE(6,3000)
      WRITE(6,9500) IFIL
900  IF((LPW20*4-LR820).EQ.0) GO TO 905
      WRITE(6,2000)
      WRITE(6,9700) IFIL
      ISTOP=1
C
C      FILE NO 21
C
905  IFIL=21
      IF(NR21-NN*NS/NDF) 910,930,920
910  WRITE(6,2000)
      WRITE(6,6000) IFIL
      ISTOP=1
      GO TO 930
920  WRITE(6,3000)
      WRITE(6,5000) IFIL
C
930  IF(LCR21-24) 940,960,950
940  WRITE(6,2000)
      WRITE(6,8000) IFIL
      ISTOP=1
      GO TO 960
950  WRITE(6,3000)
      WRITE(6,8500) IFIL
C
960  IF(LPW21-6) 970,990,980
970  WRITE(6,2000)
      WRITE(6,9000) IFIL
      ISTOP=1
      GO TO 990
980  WRITE(6,3000)
      WRITE(6,9500) IFIL
990  IF((LPW21*4-LR821).EQ.0) GO TO 995
      WRITE(6,2000)
      WRITE(6,9700) IFIL
      ISTOP=1

```



```

C
C      FILE NO 22
C      995      IF(NPBC2.EQ.0) GO TO 1050
C                IF(ILE=22)
C                IF(NP22-NPBC2) 1000,1020,1010
C      1000      WRITE(6,2000) IFILE
C                ISTOP=1
C                GO TO 1020
C      1010      WRITE(6,2000) IFILE
C                GO TO 1020
C      1020      IF(LRB22-24) 1030,1050,1040
C      1030      WRITE(6,2000) IFILE
C                ISTOP=1
C      1040      GO TO 1050
C                WRITE(6,3000) IFILE
C                WRITE(6,8500) IFILE
C
C                NUMBER OF MATERIALS
C
C      1050      IF(NMAT.LE.10) GO TO 1055
C                WRITE(6,2000) ISTOP=1
C      1055      IF((ISTP9.EQ.0).AND.(ISTP16.EQ.0)) RETURN
C                WRITE(6,3000)
C                WRITE(6,9600)
C                WRITE(6,4000)
C                CALL SKIP
C                CALL MAIN1
C
C                ENTRY ERROR2
C
C                ENTRY 'ERROR2' CHECKS THE NUMBER OF RECORDS OF FILE NO 7
C
C                IFILE=7
C                IF(NR7-MM*NN*NREC7) 1060,1080,1070
C      1060      WRITE(6,2000) IFILE
C                ISTOP=1
C                GO TO 1080
C      1070      WRITE(6,3000)

```

```

ERR03580
ERR03590
ERR03600
ERR03610
ERR03620
ERR03630
ERR03640
ERR03650
ERR03660
ERR03670
ERR03680
ERR03690
ERR03700
ERR03710
ERR03720
ERR03730
ERR03740
ERR03750
ERR03760
ERR03770
ERR03780
ERR03790
ERR03800
ERR03810
ERR03820
ERR03830
ERR03840
ERR03850
ERR03860
ERR03870
ERR03880
ERR03890
ERR03900
ERR03910
ERR03920
ERR03930
ERR03940
ERR03950
ERR03960
ERR03970
ERR03980
ERR03990
ERR04000
ERR04010
ERR04020
ERR04030
ERR04040
ERR04050

```













RETURN  
END

```

SUBROUTINE SORT
  IMPLICIT REAL*8 (A-H,O-Z)
  COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
1  NPBC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDFP,NLINE,NREC7,KEL
  COMMON /NB2/NCON(20),NBC(4)
  COMMON /B2/ELAST(6,6),COORD(3),COREL(20,3),UJNT(3),SSSS1(7),
  SSNN1(7)

```

IN THIS SUBROUTINE THE ARRAYS OF ELEMENT NODAL COORDINATES  
COREL(NPEL,NDF), ARE FORMED AND STORED ON DISK

```

REWIND 9
REVIND 19
DO 300 I=1,NEL
  READ(9) NCON
  DO 200 J=1,NPEL
    NTRACK=NCON(J)
    CALL RDISK(NTRACK)
    DO 100 K=1,NDF
      CORREL(J,K)=COORD(K)
    CONTINUE
  CONTINUE
WRITE(19) CORL
CONTINUE
RETURN
END

```

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```

SUBROUTINE MULT(A,R,C,NP,A,NCA,NCB)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION A(1),R(1),C(1)
ZRC=0.0D0
DO 100 I=1,NRA

```

MLT00029  
MLT00030  
MLT00040  
MLT00050  
MLT00060



```

100 DO 100 J=1,NCB
    IC=I+(J-1)*NRA
    C(IC)=ZRO
    DO 100 K=1,NCA
        IA=I+(K-1)*NRA
        IR=K+(J-1)*NCA
        C(IC)=C(IC)+A(
CONTINUE
RETURN
END

```

UUUUU

```

SUBROUTINE SYMINV(A,N,B,C,IFLG)
IMPLICIT REAL*8(A-H,O-Z)
DIMENSION A(1),B(1),C(1)

```

SYMMETRICAL MATRIX INVERSION

```

IFLG=0
ZRO=0.0D0
DABIGN=1.0D15
TRACE=ZRO
DO 5 I=1,N
  IAD=(I-1)*N+I
  IF(A(IAD).GF.ABIGN) GO TO 5
  TRACE=TRACE+DABS(A(IAD))
CONTINUE
APZRO=TRACE*1.0D-12
IF(APZRO.GT.1.0D-20) APZRO=1.0D-20
LP=N
DO 10 I=2,LP
  DO 10 J=I,LP
    ICOL=J+LP*(I-2)
    IROW=I+(J-1)*LP-1
    IF(A(ICOL).EQ.A(IROW)) GO TO 10
    A(ICOL)=0.5D0*(A(ICOL)+A(IROW))
    A(IROW)=A(ICOL)
CONTINUE
DO 25 I=1,N
  B(I)=ZRO
  II=(I-1)*N

```



```

DO 20 J=1,N
JJ=I+J
IF(A(JJ).GE.ABIGN) GO TO 20
B(I)=B(I)+DABS(A(JJ))
20 CONTINUE
25 ANR=ZRC
DO 30 I=1,N
ANR=DMAXI(ANR,B(I))
30 DO 145 J=1,N
NR=(I-1)*N
DO 100 J=1,N
K=NR+J
100 B(J)=A(K)
D=B(I)
IF(D.EQ.ZRO) GO TO 180
IF(DABS(D).LT.APZRO) IFLG=2
DO 110 J=1,N
C(J)=-B(J)/D
110
C
L=1
DO 130 J=1,N
M=L
DO 120 K=J,N
DB=DABS(B(J))
IF(DB.LE.1.0D-40) GO TO 115
DC=DABS(C(K))
IF(DC.LE.1.0D-40) GO TO 115
IF((DB.LE.APZRO).AND.(DC.LE.APZRO)) GO TO 115
A(L)=A(L)+B(J)*C(K)
115 CONTINUE
A(M)=A(L)
M=M+N
L=L+1
120 L=L+J
130
C
C(I)=-1.0D0/D
M=I
DO 140 J=1,N
K=NR+J
A(K)=C(J)
A(M)=C(J)
140 M=M+N
145 CONTINUE
C
NS=N*N
DO 150 J=1,NS

```





SMV00810  
 SMV00820  
 SMV00830  
 SMV00840  
 SMV00850  
 SMV00860  
 SMV00870  
 SMV00880  
 SMV00890  
 SMV00900  
 SMV00910  
 SMV00920  
 SMV00930  
 SMV00940  
 SMV00950  
 SMV00960  
 SMV00970  
 SMV00980  
 SMV00990  
 SMV01000  
 SMV01010  
 SMV01020

```

C 150 A(J)=-A(J)
    DO 165 I=1,N
      B(I)=ZRC
      II=(I-I)**N
      DO 160 J=1,N
        JJ=II+J
        IF(A(JJ).EQ.1.0D0) GO TO 160
      B(I)=B(I)+DABS(A(JJ))
    160 CONTINUE
    165 AINR=ZRC
    DO 170 I=1,N
      AINR=DMAX1(AINR,B(I))
      IF(AINR.LT.1.0D-15) AINR=1.0D0/AINR
      CNBR=AINR*AINR
    170 WRITE(6,2000) CNBR,AINR,AINR
    2000 FORMAT(5X,'CONDITION NUMBER ',5X,1PD25.16,5X,2(1PD25.16))
    180 RETURN
    END
  
```



MAI000010  
MAI000015  
MAI000020  
MAI000025  
MAI000030

SAME AS FOR QUADRATIC ELEMENTS

STOP  
END

\*\*\*\*\*

```

SUBROUTINE MAINE
IMPLICIT REAL*8 (A-H, O-Z)
COMMON /NB1/ NEL, NDPT, NPEL, NDF, NEQ, NN, MM, NS, NCOUNT, NST, NSTF, NCLD,
1 NDBC, NPBC2, NBCL, NBRCH, NMBAT, NCON21, NDEF, NLIN, NREC7, KEL
COMMON /B2/ ELAST(6,6), COORD(3), COREL(32,3), UJNT(3), SSSS1(7),
1 SSNN1(7)

```

\* SAME AS FOR QUADRATIC ELEMENTS

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\* \* \* \* \*

END

SUBROUTINE INPUT

```

      IMPLICIT REAL*8 (A-H,O-Z)
      COMMON /NBL/NEL,NDPT,NDEL,NDF,NEQ,NIL,MM,NS,NCOUNT,NST,NSTF,NCLD,
      1,NPBC,NPBC2,NBCL,NBCH,NMAT,NCON21,NCON22,NLINE,NREC7,KEL

```

I	N	P	0	0	0	2	0
I	N	P	0	0	0	3	0
I	N	P	0	0	0	4	0
I	N	P	0	0	0	5	0
I	N	P	0	0	0	6	0



UUUUUUUUUUUUUUUUUUUU

# SAME AS FOR QUADRATIC ELEMENTS

# SAME AS FOR QUADRATIC ELEMENTS

# CONNECTIVITY MATRIX

UUUU UU

SELECT THE APPROPRIATE SEGMENT (1) OR (2) DEPENDING ON WHETHER ONE OR MORE MATERIALS ARE PRESENT IN THE STRUCTURE

IF(NMAT.EQ.1) GO TO 150

(1) THIS SEGMENT IS USED FOR THE CASE OF MORE THAN ONE MATERIALS PRESENT IN THE STRUCTURE

```

DO 100 I=1,NEL
  DREAD(5,FMT),NCON1
  DWRITE(5,FMT),NCON1,NCON2,NCON21
  DWRITE(6,FMT),NCON2,NCON21
  DWRITE(9),NCON21
  DWRITE(16),NCON21

```

COMPUTATION OF HALF BANDWIDTH

```
NPPEL=MPEL-1
DO 100 J=1,NPELM
  JP=J+1
  DO 100 K=JP,NPEL
```

```

1000 DU=100-K-JP,NFEL
1100 NBAND=MAXO(NBAND,IABS(NCON(J)-NCON(K)))

```

INP00500  
INP00510  
INP00520  
INP00530  
INP00540  
INP00550  
INP00555  
INP00560  
INP00565  
INP00570  
INP00580  
INP00590  
INP00600  
INP00610  
INP00620  
INP00630  
INP00640  
INP00650  
INP00660  
INP00670  
INP00680  
INP00685  
INP00690  
INP00695  
INP00700  
INP00710  
INP00720  
INP00730  
INP00740  
INP00750  
INP00760  
INP00770  
INP00780  
INP00790



```

C      GO TO 200
C
C      (2) THIS SEGMENT IS USED FOR THE CASE OF ONE MATERIAL
150  DO 160 I=1,NEL
      READ(5,FMT) NCON1
      WRITE(6,FMT) NCON1
      READ(5,FMT) NCON2
      WRITE(6,FMT) NCON2
      WRITE(9) NCON
C
C      COMPUTATION OF HALF BANDWIDTH
      NPELM=NPEL-1
      DO 160 J=1,NPELM
        JP=J+1
        DO 160 K=JP,NPEL
          NBAND=MAXO(NBAND,IABS(NCON(J)-NCON(K)))
160  NBAND=(NBAND+1)*NDF
200  MM=(NBAND+2*(NS-1))/NS
      IF(NN.LT.MM) MM=NN
      WRITE(6,4000) NEQ,NBAND,NN,MM,NCOUNT,NSTF
C
C      SAME AS FOR QUADRATIC ELEMENTS
C
C      END
C
C      INP02390
C
C      *****
C
C      SUBROUTINE ELPROP(I)
C      IMPLICIT REAL*8(A-H,O-Z)
C      COMMON /B1/ ELDAT(10,3),CLOAD(4),BCON(3)
C      COMMON /B2/ ELAST(6,6),COORD(3),COREL(32,3),UJNT(3),SSSS1(7),
C      1SSNN1(7)
C      *****
C
C      SAME AS FOR QUADRATIC ELEMENTS
C
C      *****
C
C      INP00800
C      INP00810
C      INP00820
C      INP00830
C      INP00840
C      INP00850
C      INP00860
C      INP00865
C      INP00870
C      INP00875
C      INP00880
C      INP00890
C      INP00900
C      INP00910
C      INP00920
C      INP00930
C      INP00940
C      INP00950
C      INP00960
C      INP00970
C      INP00980
C      INP00990
C      INP01000
C      INP01010
C
C      ELP00020
C      ELP00030
C      ELP00040
C      ELP00050
C      ELP00060

```













```

COMPUTATION OF DERIVATIVES OF SHAPE FUNCTIONS WITH RESPECT
TO 'XI','ETA','ZETA' EVALUATED AT THE POINTS OF INTEGRATION
(1) CORNER NODES : XI= +1 OR -1
                     ZETA= +1 OR -1
                     ETA= +1 OR -1
                     NODE # : 1,4,7,10,21,24,27,30
DO 100 I=1,2
  II=20*(I-1)+1
  IT=II+9
  DO 100 J=1,IT,3
    XI=CORDG(J,1)
    Y1=CORDG(J,2)
    Z1=CORDG(J,3)
    W1(1,J)=DFX(X,Y,Z,X1,Y1,Z1)
    W1(2,J)=DFX(Y,X,Z,Y1,X1,Z1)
    W1(3,J)=DFX(Z,X,Y,Z1,X1,Y1)
  100
(2) MIDDLE NODES : XI= + OR - 1/3
                     ZETA= + OR - 1/3
                     ETA= + OR - 1/3
                     NODE # : 2,3,8,9,22,23,28,29
DO 200 K=1,2
  IK=20*(K-1)
  DO 200 I=1,2
    II=6*(I-1)+2+IK
    IT=II+1
    DO 200 J=1,IT
      XI=CORDG(J,1)
      Y1=CORDG(J,2)
      Z1=CORDG(J,3)
      W1(1,J)=DX(X,Y,Z,X1,Y1,Z1)
      W1(2,J)=DY(X,Y,Z,X1,Y1,Z1)
      W1(3,J)=DZ(X,Y,Z,X1,Y1,Z1)
    200
(3) MIDDLE NODES : XI= + OR - 1/3
                     ZETA= + OR - 1/3
                     ETA= + OR - 1/3
                     NODE # : 5,6,11,12,25,26,31,32
DO 300 K=1,2
  IK=20*(K-1)
  DO 300 I=1,2
    II=6*(I-1)+5+IK
    IT=II+1
    DO 300 J=1,IT
      XI=CORDG(J,1)
      Y1=CORDG(J,2)
      Z1=CORDG(J,3)
    300

```

```

CCCCCCCC
CCCC
CCCC

```



```

3000 W1(1,J)=DY(Y, Z,Y1,X1,Z1)
      W1(2,J)=DX(Y,X,Z,Y1,X1,Z1)
      W1(3,J)=DZ(Y,X, Y1,X1,Z1)

      (4) MIDDLE NODES
            NODE #
DO 400 K=1,2
IK=4*(K-1)
I1=13+IK
IT=I1+3
DO 400 J=1,I,IT
X1=CORDG(J,1)
Y1=CORDG(J,2)
Z1=CORDG(J,3)
W1(1,J)=DZ(Z,Y, Z1,Y1,X1)
W1(2,J)=DY(Z,X,Z1,Y1,X1)
W1(3,J)=DX(Z,Y,X,Z1,Y1,X1)
4000

```

RETURN  
END

[illegible]

```

SUBROUTINE MERGE
  IMPLICIT REAL*8 (A-H,O-Z)
  COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
  & NPB2,NBCL,NBCH,NMAT,NCON21,NDF2,NLINE,NREC7,KEL
  COMMON /NB2/ NCON(32),NBC(4)
  COMMON /B3/ AK1(96,96),AK2(96,96),AK3(96,96),RB1(96),RB2(96),
  & RB3(96)
  DIMENSION A(96,96),S(96,96),B(9216),I(9216),LM(32)

```

# SAME AS FOR QUADRATIC ELEMENTS





[illegible]







CCCCC

\* SAME AS FOR QUADRATIC ELEMENTS \*

END

DSP01320

CCCCC

\*\*\*\*\*

C

SUBROUTINE STRESS(I)

IMPLICIT REAL\*8(A-H,O-Z)  
COMMON /NB1/NEL,NDPT,NDEL,NDF,NEQ,NN,4M,NS,NCOUNT,NST,NSTF,NCLD,  
1NPBC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDF,NLINE,NREC7,KEL  
COMMON /NB2/NCON(32),NRC(4)  
COMMON /B2/ ELAST(6,6),COORD(3),COREL(32,3),UJNT(3),SSSS1(7),  
1SSNN1(7)  
COMMON /R3/ AK1(96,96),AK2(96,96),AK3(96,96),RB1(96),RB2(96),  
1RB3(96)  
1DIMENSION SNELM(32,6),UEL(96),SSELN(32,6),SSNN(6),B(6,96),SSSS(6),  
1CORDG(32,3),LM(32)  
1EQUIVALENCE (AK3(1,1),B(1)),(AK3(1,12),SNELM(1)),(AK3(1,15),  
1SSELN(1)),(AK3(1,17),SSNN(1)),(AK3(7,17),SSSS(1)),  
2(UEL(1),RB1(1))  
11.0D0,-1.0D0,0.3333333333333333,0.3333333333333333,-1.0D0,  
1-1.0D0,-1.0D0,-1.0D0,-1.0D0,1.0D0,1.0D0,1.0D0,1.0D0,  
21.0D0,1.0D0,1.0D0,1.0D0,-1.0D0,1.0D0,1.0D0,-1.0D0,  
31.0D0,0.3333333333333333,-0.3333333333333333,1.0D0,-1.0D0,  
4-1.0D0,-0.3333333333333333,0.3333333333333333,1.0D0,1.0D0,  
51.0D0,1.0D0,1.0D0,0.3333333333333333,-0.3333333333333333,  
6-1.0D0,-1.0D0,-1.0D0,-1.0D0,1.0D0,1.0D0,-1.0D0,1.0D0,  
71.0D0,1.0D0,1.0D0,-1.0D0,-1.0D0,1.0D0,1.0D0,1.0D0,  
81.0D0,1.0D0,0.3333333333333333,-0.3333333333333333,-1.0D0,  
9-1.0D0,-1.0D0,-0.3333333333333333,0.3333333333333333,1.0D0,  
A1.0D0,1.0D0,1.0D0,1.0D0,1.0D0,1.0D0,1.0D0,1.0D0,  
B0.3333333333333333,0.3333333333333333,0.3333333333333333,  
C0.3333333333333333,-0.3333333333333333,-0.3333333333333333,  
D33333,-0.3333333333333333,-1.0D0,-1.0D0,-1.0D0,-1.0D0,  
E,-1.0D0,-1.0D0,-1.0D0,-1.0D0,-1.0D0,-1.0D0/  
STR000920  
STR000930  
STR000940  
STR000950  
STR000960  
STR000970  
STR000980  
STR000990  
STR001000  
STR001010  
STR001020  
STR001030  
STR001040  
STR001050  
STR001060  
STR001070  
STR001080  
STR001090  
STR001100  
STR001110  
STR001120  
STR001130  
STR001140  
STR001150  
STR001160  
STR001170  
STR001180  
STR001190  
STR001200  
STR001210  
STR001220  
STR001230  
STR001240  
STR001250  
STR001260  
STR001270  
STR001280  
STR001290  
STR001300  
STR001310

CCCCC

\* SAME AS FOR QUADRATIC ELEMENTS \*

















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SMV01010  
SMV01020

\*

\*

RETURN  
END

C





LISTING FOR AUXILIARY PROGRAM JCL

153



CC

FILE NO 8

NREC8=4  
IF(NPEL.EQ.32) NREC8=12  
NRD1(2)=NEL\*NREC8  
LRWD1(2)=NSTF\*2/NREC8  
LRBD1(2)=LRWD1(2)\*4

JCL00490  
JCL00500  
JCL00510  
JCL00520  
JCL00530  
JCL00540  
JCL00550  
JCL00560  
JCL00570  
JCL00580  
JCL00590  
JCL00600  
JCL00610  
JCL00620  
JCL00630  
JCL00640  
JCL00650  
JCL00660  
JCL00670  
JCL00680  
JCL00690  
JCL00700  
JCL00710  
JCL00720  
JCL00730  
JCL00740  
JCL00750  
JCL00760  
JCL00770  
JCL00780  
JCL00790  
JCL00800  
JCL00810  
JCL00820  
JCL00830  
JCL00840  
JCL00850  
JCL00860  
JCL00870  
JCL00880  
JCL00890  
JCL00900  
JCL00910  
JCL00920  
JCL00930  
JCL00940  
JCL00950  
JCL00960

CC

FILE NO 10

NRD2(1)=NDPT  
LRWD2(1)=6  
LRBD2(1)=LRWD2(1)\*4

CC

FILE NO 12

NRD2(2)=NDPT  
LRWD2(2)=14  
LRBD2(2)=LRWD2(2)\*4

CC

FILE NO 13

NRD2(3)=NDPT  
LRWD2(3)=14  
LRBD2(3)=LRWD2(3)\*4

CC

FILE NO 20

NRD2(4)=NN  
LRWD2(4)=NS\*2  
LRBD2(4)=LRWD2(4)\*4

CC

FILE NO 21

NRD2(5)=NN\*NS/NDF  
LRWD2(5)=6  
LRBD2(5)=24

CC

(2) SEQUENTIAL FILES

CC

FILE NO 9

NRD1(3)=NEL  
LRBD1(3)=128  
IF(NPEL.EQ.20) LRBD1(3)=80

C



FILES NO 14,15,16,17,19,22

```
NRS(1)=NCLD
NRS(2)=NPBC
NRS(3)=NEL
NRS(4)=NPBC
NRS(5)=NEL
NRS(6)=NPBC2
IF(NMAT.GT.1) GO TO 12
LRBS(2)=0
LRBS(3)=0
12 IF(NPBC2.GT.0) GO TO 15
NRS(6)=0
LRBS(6)=0
```

CONTROL CARDS

```
15 WRITE(6,8000)
WRITE(7,8000)
WRITE(6,9600)
```

FILES NO 7,8,9

```
DO 30 I=1,3
NR=NRD1(I)
LRB=LRBD1(I)
CALL EXDIC(NR,4,0,0,IER,ANR,ALPHA)
CALL EXDIC(LRB,4,0,0,IER,ALRB,ALPHA)
REWIND 8
WRITE(8,2000) ANR
WRITE(8,2000) ALRB
REWIND 8
READ(8,3000) NRA
READ(8,3000) LRBA
DO 20 J=1,4
IF(NRA(J).EQ.IBL) NRA(J)=IZR
IF(LRBA(J).EQ.IBL) LRBA(J)=IZR
```

```
20 CONTINUE
WRITE(6,4000) NFD1(I),LRBA,NRA
WRITE(7,4100) NFD1(I),LRBA,NRA
30 CONTINUE
```

FILES NO 10,12,13,20,21

```
DO 50 I=1,5
NR=NRD2(I)
```

JCL00970  
JCL00980  
JCL00990  
JCL01000  
JCL01010  
JCL01020  
JCL01030  
JCL01040  
JCL01050  
JCL01060  
JCL01070  
JCL01080  
JCL01090  
JCL01100  
JCL01110  
JCL01120  
JCL01130  
JCL01140  
JCL01150  
JCL01160  
JCL01170  
JCL01180  
JCL01190  
JCL01200  
JCL01210  
JCL01220  
JCL01230  
JCL01240  
JCL01250  
JCL01260  
JCL01270  
JCL01280  
JCL01290  
JCL01300  
JCL01310  
JCL01320  
JCL01330  
JCL01340  
JCL01350  
JCL01360  
JCL01370  
JCL01380  
JCL01390  
JCL01400  
JCL01410  
JCL01420  
JCL01430  
JCL01440



```

LRB=L RBD2(I)
CALL EXDIC(NR,4,0,0,IER,ANR,ALPHA)
CALL EXDIC(LRB,4,0,0,IER,ALRB,ALPHA)
REWIND 8
WRITE(8,2000) ANR
WRITE(8,2000) ALRB
REWIND 8
READ(8,3000) NRA
READ(8,3000) LRBA
DO 40 J=1,4
IF(NRA(J).EQ.1RL) NRA(J)=IZR
IF(LRBA(J).EQ.1BL) LRBA(J)=IZR
CONTINUE
40 WRITE(6,5000) NFD2(I),LRBA,NRA
   WRITE(7,5100) NFD2(I),LRBA,NRA
50 CONTINUE

      FILES NO 14,15,16,17,22

DO 70 I=1,6
IF((I.EQ.3).AND.(NMAT.EQ.1)) GO TO 70
IF((I.EQ.6).AND.(NPBC2.EQ.0)) GO TO 70
NR=NRS(I)
LRB=L RBS(I)
CALL EXDIC(NP,4,0,0,IER,ANR,ALPHA)
CALL EXDIC(LRB,4,0,0,IEP,ALRB,ALPHA)
REWIND 8
WRITE(8,2000) ANR
WRITE(8,2000) ALRB
REWIND 8
READ(8,3000) NRA
READ(8,3000) LRBA
DO 60 J=1,4
IF(NRA(J).EQ.1BL) NRA(J)=IZR
IF(LRBA(J).EQ.1BL) LRBA(J)=IZR
CONTINUE
60 WRITE(6,5000) NFS(I),LRBA,NRA
   WRITE(7,5100) NFS(I),LRBA,NRA
70 CONTINUE

      DEFINE FILE STATEMENTS

120 WRITE(6,9500)
    WRITE(6,9000)
    WRITE(7,9000)
    WRITE(6,9600)

```





```

C
IC2=2
IC3=3
ICD1=150
ICD2=160
ICD3=170
ICD4=180
IF(NPEL.EQ.20) GO TO 130
ICD1=190
ICD2=200
ICD3=210
ICD4=220
130 WRITE(6,9500) NFD1(1),NRD1(1),LRWD1(1),NFD1(1),NFD1(2),NRD1(2),
A,LRWD1(2),NFD1(2),NFD1(2),NFD1(2),LRWD2(1),NFD2(1),ICD2
WRITE(6,9600) IC2,NFD2(2),NRD2(2),LRWD2(2),NFD2(2),NFD2(3),NRD2(3)
A,LRWD2(3),NFD2(3),ICD3
WRITE(6,9700) IC3,NFD2(4),LRWD2(4),NFD2(4),NFD2(5),NRD2(5)
A,LRWD2(5),NFD2(5),ICD4
WRITE(7,9800) NFD1(1),NRD1(1),LRWD1(1),NFD1(1),NFD1(2),NRD1(2),
A,LRWD1(2),NFD1(2),NFD1(2),NFD1(2),LRWD2(1),NFD2(1),ICD2
WRITE(7,9900) IC2,NFD2(2),NRD2(2),LRWD2(2),NFD2(2),NFD2(3),NRD2(3)
A,LRWD2(3),NFD2(3),ICD3
WRITE(7,9920) IC3,NFD2(4),LRWD2(4),NFD2(4),NFD2(5),NRD2(5),
A,LRWD2(5),NFD2(5),ICD4
*****
C
C
C
C
GENERAL FILE DATA
WRITE(6,9500)
WRITE(6,9800)
WRITE(6,9600)
WRITE(7,9900)
WRITE(6,9910)
WRITE(6,9900) (NRD1(1),I=1,3),(NRD2(1),I=1,2),(LRBD1(1),I=1,3),
1(LRBD2(1),I=1,2),(LRWD1(1),I=1,2),(LRWD2(1),I=1,2),NREC7,
2NRD2(3),(NRS(1),I=1,4),LRBD2(3),(LRBS(1),I=1,4),LRWD2(3),NRS(5),
3(NRD2(1),I=4,5),NRS(6),LRBS(5),(LRBD2(1),I=4,5),LRBS(6),
4(LRWD2(1),I=4,5)
WRITE(6,9920)
WRITE(7,9500) NRD1(1),LRPD1(1),LRWD1(1),NREC7,NRD1(2),LRBD1(2),
1LRWD1(2),NRD1(3),LRBD1(3),
2(LNRD2(1),LRWD2(1),I=1,3),(NRS(I),LPBS(I),I=1,5),(NRD2(I),
3LRBD2(1),LRWD2(1),I=4,5),NRS(6),LRBS(6)
GO TO 2
C
C
C
C
JCL01930
JCL01940
JCL01950
JCL01960
JCL01970
JCL01980
JCL01990
JCL02000
JCL02010
JCL02020
JCL02030
JCL02040
JCL02050
JCL02060
JCL02070
JCL02080
JCL02090
JCL02100
JCL02110
JCL02120
JCL02130
JCL02140
JCL02150
JCL02160
JCL02170
JCL02180
JCL02190
JCL02200
JCL02210
JCL02220
JCL02230
JCL02240
JCL02250
JCL02260
JCL02270
JCL02280
JCL02290
JCL02300
JCL02310
JCL02320
JCL02330
JCL02340
JCL02350
JCL02360
JCL02370
JCL02380
JCL02390
JCL02400

```







```

1, I4,4X,'NR12','=',I4/'',I4,4X,'LRB8','=',I4,4X,'LRB10','=',I4,4X,
25X,'LRB7','=',I4,4X,'LRW7','=',I4,4X,'LRW8','=',I4,18X,'LRW10','=',I4,4X,
3,'LRB12','=',I4/'',5X,'NREC7','=',I4/'',
4,'LRB12','=',I4/'',I4,4X,'NR14','=',I4,4X,'NR15','=',I4,4X,
55X,'NR13','=',I4/'',I4,4X,'NR14','=',I4,4X,'NR15','=',I4,4X,
6,'NR17','=',I4/'',I4,4X,'LRB14','=',I4,4X,'LRB15','=',I4,4X,
75X,'LRB13','=',I4,4X,'LRB14','=',I4,4X,'LRB15','=',I4,4X,
8,'LRB17','=',I4/'',I4,4X,'NR20','=',I4,4X,'NR21','=',I4/'',
95X,'LRW13','=',I4,4X,'NR20','=',I4,4X,'NR21','=',I4/'',
A5X,'NR19','=',I4,4X,'LRB20','=',I4,4X,'LRB21','=',I4,4X,'NR22','=',I4/'',
B5X,'LRB19','=',I4,4X,'LRW21','=',I4/'',
C19X,'LRW20','=',I4,4X,'NRX','=',NUMBER OF RECORDS OF FILE X/'',
9910 FORMAT(1,5X,'LENGTH OF RECORD OF FILE X IN BYTES',/,
25X,'LRBX' = -1 - IF DATA OF FILES NO 16 OR 22 ARE ZERO THESE
35X,'LRWX' = -1 - IF DATA OF FILES NO 16 OR 22 ARE ZERO THESE
9920 FORMAT(1,5X,'NOTE : IF DATA OF FILES NO 16 OR 22 ARE ZERO THESE
1 FILES ARE NOT REQUIRED')
STOP
END

```

LEXIC

[illegible]



SLOWLIM  
 WORD  
 IER  
 MAX1  
 MAX  
 MIN1  
 MIN  
 MAX2  
 MIN2  
 NEXT  
 DC  
 DC  
 DC  
 DC  
 DC  
 DC  
 DC  
 DC  
 LTR  
 BZ  
 BCT  
 LTR  
 BZ  
 LA  
 ST  
 BC  
 LA  
 CR  
 BNE  
 MVC  
 MVC  
 MVC  
 L  
 LR  
 BC  
 MVC  
 LA  
 ST  
 BC  
 L  
 LR  
 BC  
 MVC  
 SRCT  
 BCT  
 ST  
 BC  
 MVI  
 LTR  
 BC  
 MVI  
 SRCT  
 BCT  
 MR  
 LA  
 NONNEG

X'404040404060F9F9F9'  
 X'404040404040404040'  
 X'00000000'  
 X'05F5E0FF'  
 X'05F5E0FF'  
 X'FF676981'  
 X'FF676981'  
 X'0000270F'  
 X'FFFFFC19'  
 6,4  
 JTI SOK  
 6,\*+4  
 6,6  
 JTI SOK  
 0,2  
 0,IER  
 15,RETURN  
 8,4  
 3,8  
 EIGHTCAR  
 MAX(4),MAX2  
 MIN(4),MIN2  
 SLOWLIM  
 0,MAX  
 7,2  
 7,0  
 13,NOTTOBIG  
 WORD(8),UPLIM  
 0,1  
 0,IER  
 15,RETURN  
 0,MIN  
 7,2  
 7,0  
 11,INRANGE  
 WORD(8),LOWLIM  
 0,0  
 0,\*+4  
 0,IER  
 15,RETURN  
 7,FLAG+1,X'01'  
 7,2  
 11,NONNEG  
 1,FLAG+1,X'00'  
 1,1  
 1,\*+4  
 1,1  
 6,1  
 8,4

C0920  
 C0930  
 C0940  
 C0950  
 C0960  
 C0970  
 C0980  
 C0990  
 C1000  
 C1010  
 C1020  
 C1030  
 C1040  
 C1050  
 C1060  
 C1070  
 C1080  
 C1090  
 C1100  
 C1110  
 C1120  
 C1130  
 C1140  
 C1150  
 C1160  
 C1170  
 C1180  
 C1190  
 C1200  
 C1210  
 C1220  
 C1230  
 C1240  
 C1250  
 C1260  
 C1270  
 C1280  
 C1290  
 C1300  
 C1310  
 C1320  
 C1330  
 C1340  
 C1350  
 C1360  
 C1370  
 C1380  
 C1390





LA	1,WORD+7
LR	11,1
LTR	0,10
BC	7,7
MVC	7,NOTZERO
BC	WORD+7(1),ALPHA+1
SR	15,RETURN
DR	6,6
DR	6,0
AH	6,ALPHA
STH	6,TEMP-1
MVC	0(1,1),TEMP
BCT	1,*+4
LTR	7,7
BC	7,NOTZERO
LH	10,FLAG
LTR	10,10
BNZ	NOSIGN
MVC	0(1,1),MINUS
BCT	1,*+4
LR	12,11
LR	12,1
LR	10,3
SR	10,12
BZ	10,RETURN
LTR	4,4
BZ	RTJSTFD
LA	1,1(0,1)
LR	14,10
SR	14,12
LR	11,14
MVC	0(1,14),0(1)
MVC	0(1,1),BLANK
LA	1(1,1),1
LA	14,1(0,14)
BCT	11,SHIFTL
BCT	1,*+4
SR	14,*+4
SR	10,5
BM	10,RETURN
LTR	1,RETURN
BNP	1,14
AR	1,14
LR	14,12
LR	14,11
MVC	0(1,1),0(1)
BCT	0(1,1),BLANK
BCT	11,*+4



BCT		1,*+4
BCT		114,SECSHFTL
BC		115,RETURN
LTR		111,5
BNP		110,RETURN
SR		110,11
BM		11,RETURN
LA		11,1(0,1)
LR		114,1
SR		114,11
LR		11,11
MVC		10,112
MVC		0(1,1),0(11)
LA		0(1,1),BLANK
LA		11,1(0,11)
BCT		11,1(7),SHFTR
LA		110,SECSS
LA		1,ADDRESS
MVC		1,4(0,1)
LA		1,0(0,1)IER
MVC		1,0(4,1)ESS
LA		1,8(0,1)
MVC		1,0(0,1)WORD+4
LA		1,0(4,1)ESS
S		1,12(0,1)
MVC		1,0(0,1)
LM		1,EIGHTY
MVI		0(8,1),WORD
MVC		112(13),12(113)
MVC		1EF+2(1),X'FF'
MVC		WORD(1),IER
MVC		WORD(1),BLANK
MVC		WORD+1(7),LOWLMI
MVC		MAX(4),MAX1
MVC		MIN(4),MINI
BCR		15,14
DS		OF'8000000'
DC		X'0001'
DC		X'00'
DC		X'00'
DC		F
END		



## APPENDIX I

## LISTINGS OF INTEGRATION SUBROUTINES

### A. QUADRATIC ELEMENTS - TWO GAUSS POINTS

```

C SUBROUTINE CUB
C
C IMPLICIT REAL*8(A-H,O-Z)
C COMMON /NB1/NEL,NDPT,NPEL,NDE,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
1NPBC,NPBC2,NBCL,NBCN,NMAT,NCON21,NDFP,NLINE,NREC7,KEL
C COMMON /B3/ AK1(60,60),AK2(60,60),AK3(60,60),RB1(60),RB2(60),
1RB3(60)
C DIMENSION STK(60,60),AK(60,60),B(6,60)
C EQUIVALENCE (STK(1,1),AK1(1,1)),(AK(1,1),AK2(1,1)),(B(1,1),
1AK3(1,1))
C DIMENSION XI(2)
C DATA XI/O.5773502691896258,-0.5773502691896258/
C
C DO 100 I=1,NST
C DO 100 J=1,NST
C STK(I,J)=0.0D0
C
C 100
C
C      SELECTION OF POINTS OF INTEGRATION AND FORMATION OF
C      INTEGRANT BY SUBROUTINE 'FORMK'
C
C DO 200 I=1,2
C X=XI(I)
C DO 200 J=1,2
C Y=XI(J)
C DO 200 K=1,2
C Z=XI(K)
C
C CALL FORMK(X,Y,Z,1)
C
C      INTEGRATION BY GAUSSIAN FORMULA AND FORMATION OF ELEMENT
C      STIFFNESS MATRIX
C
C DO 200 L=1,NST
C DO 200 M=L,NST
C STK(L,M)=STK(L,M)+ AK(L,M)
C
C 200 DO 300 I=1,NST
C DO 300 J=I,NST
C STK(J,I)=STK(I,J)
C
C RETURN
C END
C

```



[illegible]

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# C. QUADRATIC ELEMENTS - FOUR GAUSS POINTS

```

SUBROUTINE CUB
IMPLICIT REAL*8(A-H,O-Z)
COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
1NPRC,NPRC2,NBCH,NMAT,NCON21,NDFP,NLINE,NPFC7,KEL
COMMON /B3/ AK1(60,60),AK2(60,60),AK3(60,60),RB1(60),RB2(60),
1RB3(60)
DIMENSION STK(60,60),AK(60,60),B(6,60),XI(4),AI(4),AIA(4,4,4)
EQUIVALENCE (STK(1,1),AK1(1,1)),(AK(1,1),AK2(1,1)),(B(1,1),
1AK3(1,1))
DATA XI/0.8611363115940526, 0.3399810435848563,
1-0.3399810435848563,-0.8611363115940526/
1 DATA AI/0.3478548451374539,0.6521451548625461,
1 0.6521451548625461,0.3478548451374539/
DO 100 I=1,NST
DO 100 J=1,NST
100 STK(I,J)=0.000
CC
CC
COMPUTATION OF PRODUCTS OF WEIGHTS
DO 150 I=1,4
DO 150 J=1,4
DO 150 K=1,4
150 AIA(I,J,K)=AI(I)*AI(J)*AI(K)
CCCC
SELECTION OF POINTS OF INTEGRATION AND FORMATION OF
INTEGRANT BY SUBROUTINE 'FORMK'
DO 250 I=1,4
X=XI(I)
DO 250 J=1,4
Y=XI(J)
DO 250 K=1,4
Z=XI(K)
CALL FORMK(X,Y,Z,1)
CCCC
INTEGRATION BY GAUSSIAN FORMULA AND FORMATION OF ELEMENT
STIFFNESS MATRIX
DO 200 L=1,NST
DO 200 M=L,NST
200 STK(L,M)=STK(L,M)+AIA(I,J,K)*AK(L,M)
250 CONTINUE
DO 300 I=1,NST
DO 300 J=I,NST
300 STK(J,I)=STK(I,J)
RETURN

```



#### D. CUBIC ELEMENTS - THREE GAUSS POINTS

```

SUBROUTINE CUB
  IMPLICIT REAL*8(A-H,C-Z)
  COMMON /NB1/NEL,NDPT,NDEL,NDF,NEQ,NN,MM,NS,NCOUNT,NST,NSTF,NCLD,
  1NPRC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDFP,NLINE,NRECT,KEL
  1COMMON /B3/ AK1(96,96),AK2(96,96),AK3(96,96),RB1(96),RB2(96),
  1RB3(96),
  1DIMENSION STK(96,96),AK(96,96),R(6,96),XI(3),AI(3),AIA(3,3,3),
  1EQUIVALENCE (STK(1,1),AK1(1,1)),(AK(1,1),AK2(1,1)),(B(1,1),
  1AK3(1,1))
  1DATA XI/0.7745966692414834,0.0DC,-0.7745966692414834/
  1DATA AI/C.5555555555555556,0.888888888888889,0.5555555555555556/
  1DO 100 I=1,NST
  1DO 100 J=1,NST
  100 STK(I,J)=0.0DO

      COMPUTATION OF PRODUCTS OF WEIGHTS

      DO 150 I=1,3
      DO 150 J=1,3
      DO 150 K=1,3
      150 AIA(I,J,K)=AI(I)*AI(J)*AI(K)

      SELECTION OF POINTS OF INTEGRATION AND FORMATION OF
      INTEGRANT BY SUBROUTINE FORMK.

      DO 250 I=1,3
      X=XI(I)
      DO 250 J=1,3
      Y=XI(J)
      DO 250 K=1,3
      Z=XI(K)

      CALL FORMK(X,Y,Z,1)

      INTEGRATION BY GAUSSIAN FORMULA AND FORMATION OF ELEMENT
      STIFFNESS MATRIX

      DO 200 L=1,NST
      DO 200 M=L,NST
      STK(L,M)=STK(L,M)+AIA(I,J,K)*AK(L,M)
      200 CONTINUE
      DO 300 I=1,NST
      DO 300 J=I,NST
      STK(J,I)=STK(I,J)
      300 RETURN
      END

```







# F. CUBIC ELEMENTS - FIVE GAUSS POINTS

```

SUBROUTINE CUB
  IMPLICIT REAL*8(A-H,O-Z)
  COMMON /NB1/NEL,NDPT,NPEL,NDF,NEQ,NN,MM,NS,NCCUNT,NST,NSTF,NCLD,
  INPRC,NPBC2,NBCL,NBCH,NMAT,NCON21,NDFP,NLINE,NRECT,KEL
  COMMON /B3/ AK1(96,96),AK2(96,96),AK3(96,96),RB1(96),RB2(96),
  IRB3(96)
  DIMENSION STK(96,96),AK(96,96),B(6,96)
  EQUIVALENCE (STK(1,1),AK1(1,1)),(AK(1,1),AK2(1,1)),(B(1,1),
  1AK3(1,1))
  DIMENSION XI(5),AIA(5,5,5)
  DATA XI/0.9061798459386640,0.5384693101056831,C.0D0,-0.53846931010
  156831,-0.9061798459386640/
  DATA AIA/I/O.2369268850561891,0.4786286704993665,0.5688888888888889,0
  1.4786286704993665,0.2369268850561891/
  DO 100 I=1,NST
  DO 100 J=1,NST
  100 STK(I,J)=0.0D0
  CCC
  COMPUTATION OF PRODUCTS OF WEIGHTS
  DO 150 I=1,5
  DO 150 J=1,5
  DO 150 K=1,5
  150 AIA(I,J,K)=AI(I)*AI(J)*AI(K)
  CCC
  SELECTION OF POINTS OF INTEGRATION AND FORMATION OF
  INTEGRANT BY SUBROUTINE FORMK.
  DO 200 I=1,5
  X=XI(I)
  DO 200 J=1,5
  Y=XI(J)
  DO 200 K=1,5
  Z=XI(K)
  CALL FORMK(X,Y,Z,1)
  CCC
  INTEGRATION BY GAUSSIAN FORMULA AND FORMATION OF ELEMENT
  STIFFNESS MATRIX
  DO 200 L=1,NST
  DO 200 M=L,NST
  DO 300 I=1,NST
  DO 300 J=I,NST
  200 STK(L,M)=STK(L,M)+AIA(I,J,K)*AK(L,M)
  300 STK(J,I)=STK(I,J)
  K=RETURN

```





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ORIGINATING ACTIVITY (Corporate author)		2a. REPORT SECURITY CLASSIFICATION	
Naval Postgraduate School Monterey, California 93940		Unclassified	
		2b. GROUP	
REPORT TITLE			
A GENERAL PURPOSE THREE DIMENSIONAL STRESS ANALYSIS PROGRAM			
DESCRIPTIVE NOTES (Type of report and, inclusive dates)			
Master's Thesis; December 1971			
AUTHOR(S) (First name, middle initial, last name)			
Emmanuel Leonidas Lieutenant Commander, Hellenic Navy			
REPORT DATE		7a. TOTAL NO. OF PAGES	7b. NO. OF REFS
December 1971		173	8
1. CONTRACT OR GRANT NO.		9a. ORIGINATOR'S REPORT NUMBER(S)	
2. PROJECT NO.			
		9b. OTHER REPORT NO(S) (Any other numbers that may be assigned this report)	
3. DISTRIBUTION STATEMENT			
Approved for public release; distribution unlimited.			
11. SUPPLEMENTARY NOTES		12. SPONSORING MILITARY ACTIVITY	
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4. ABSTRACT			
<p>A computerized solution for three dimensional stress analysis was developed. The solution was based on the finite element technique employing twenty and thirty-two nodal point three dimensional isoparametric elements. The solution is applicable to linear elastic structures composed of homogeneous and isotropic materials subjected to static loading under constant temperature. Deformed states are restricted to small displacements and displacement gradients. The program is coded in standard FORTRAN IV and is machine independent except for one subroutine which may be adjusted to specific installations. Independence of core space requirements is achieved by use of direct access storage facilities.</p>			





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